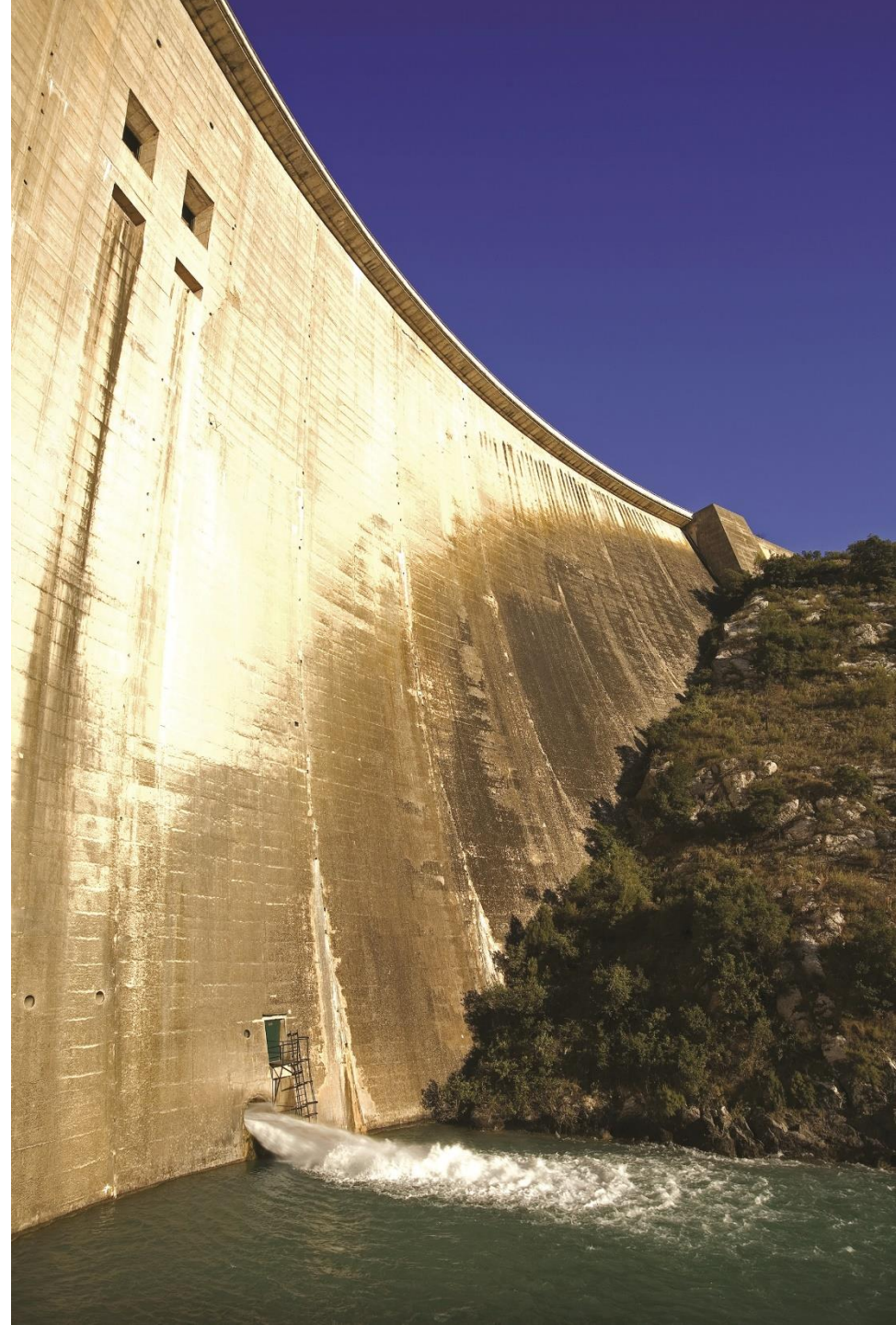


LE BARRAGE FAIT PEAU NEUVE



BARRAGE RÉNOVÉ, PATRIMOINE PRÉSERVÉ



1

Context

2

Preliminary studies

3

Numerical Modelling

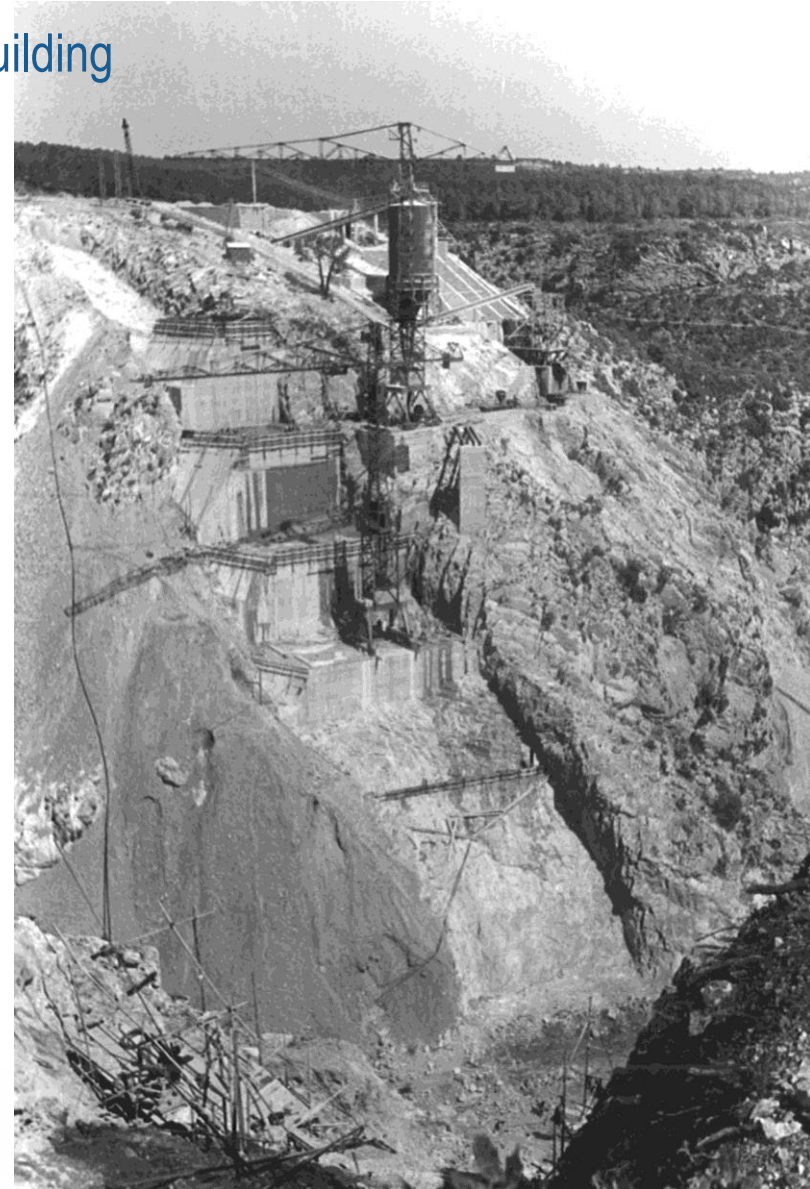
4

Rehabilitation works

01

CONTEXT

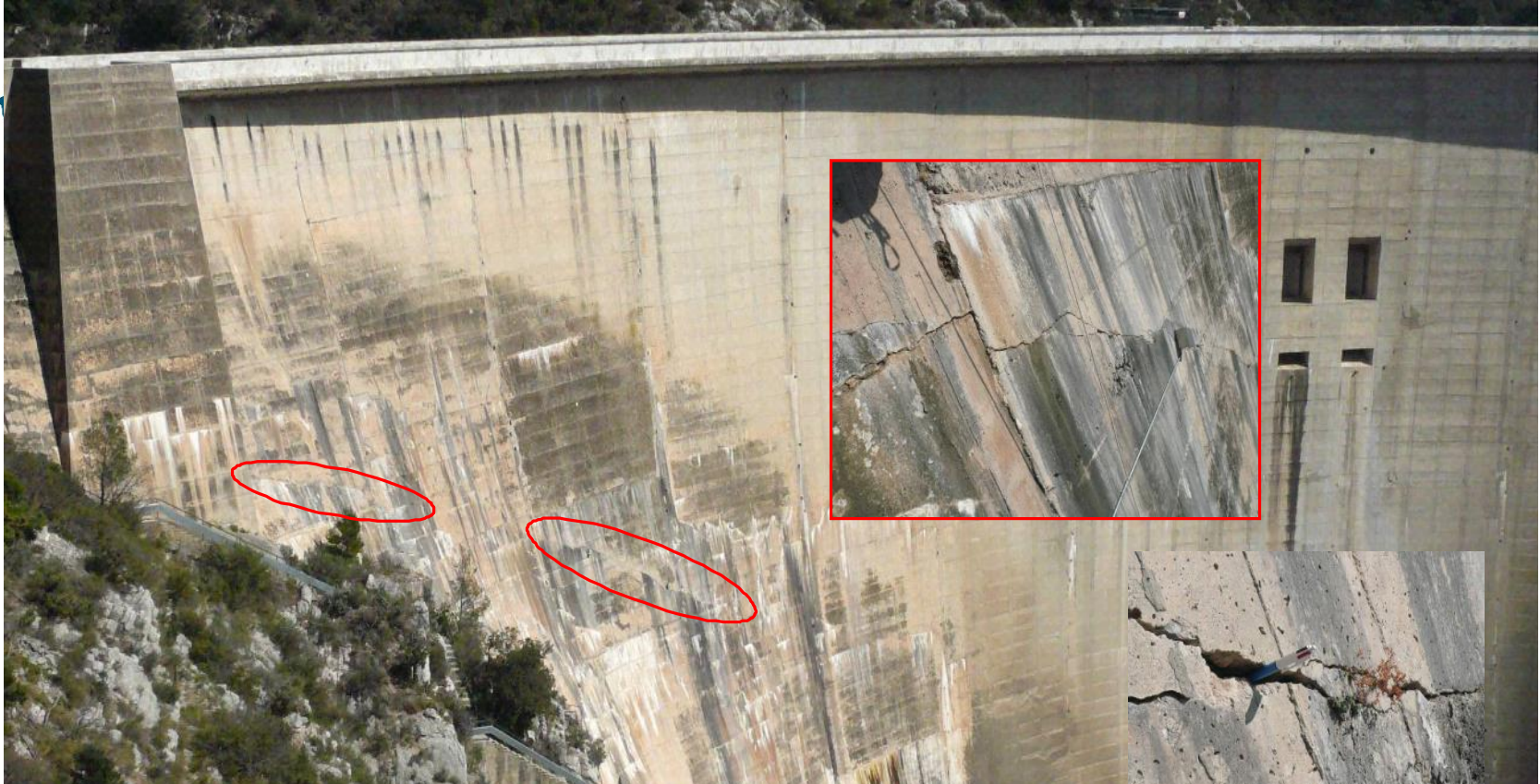
1947 – 1951 : Building



- 1963: The SCP takes possession of the bimont Dam
- Only 3 years after Malpasset disaster
- Numerous cracks observed on the second plot and on the right bank of the dam



Bimont



Partager l'eau,
construire l'avenir

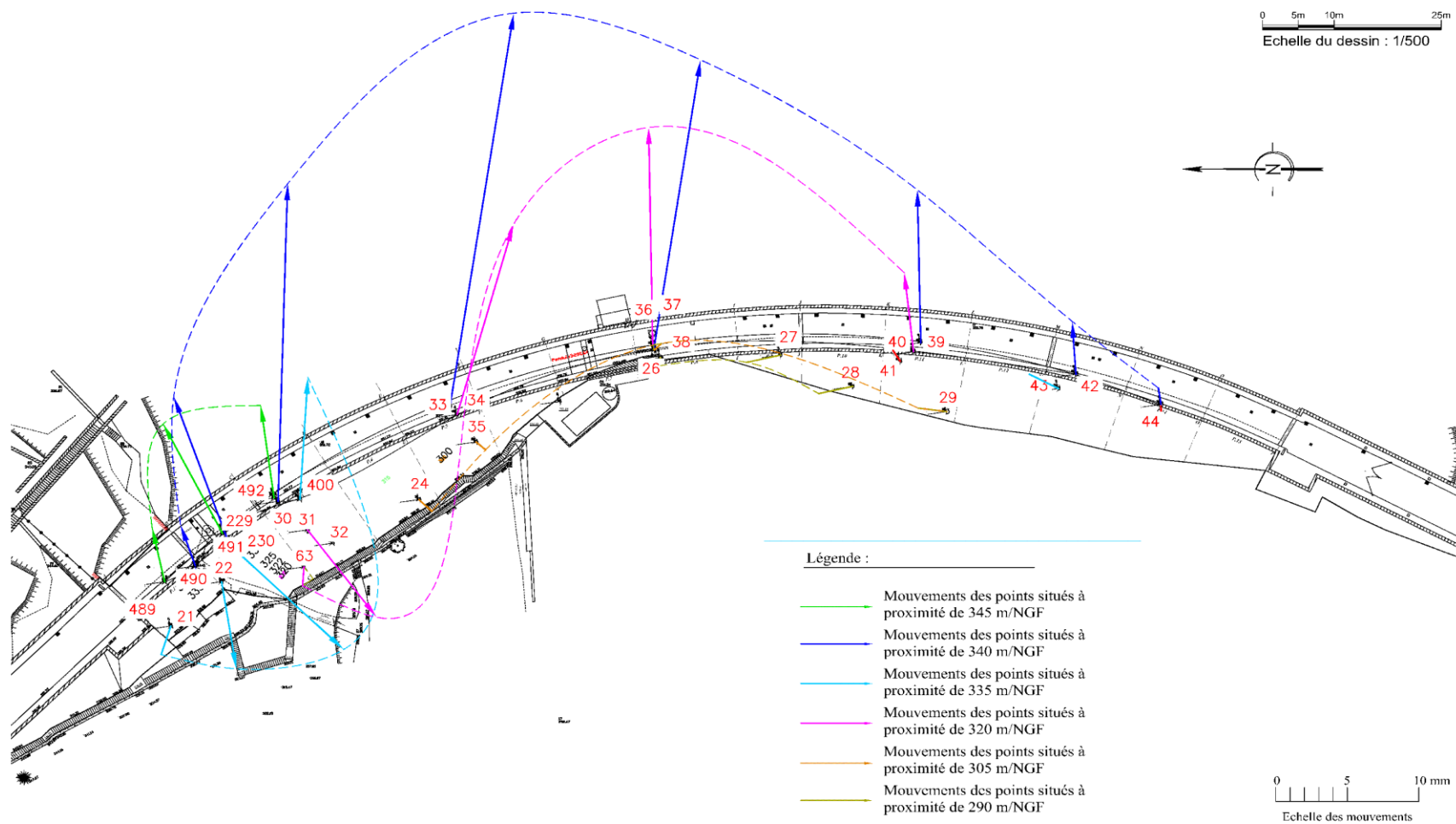
TRACTEBEL
ENGIE

- The geology was first suspected.
- Numerous fields of investigations:
 - Complementary geological studies
 - Gallery cutted into the rock foundation of the second cantilever
 - Additionnal monitoring instruments: 2 plumbelines, fissurometers, Topographic survey
 - Mapping of the cracks network
 - Several numerical models

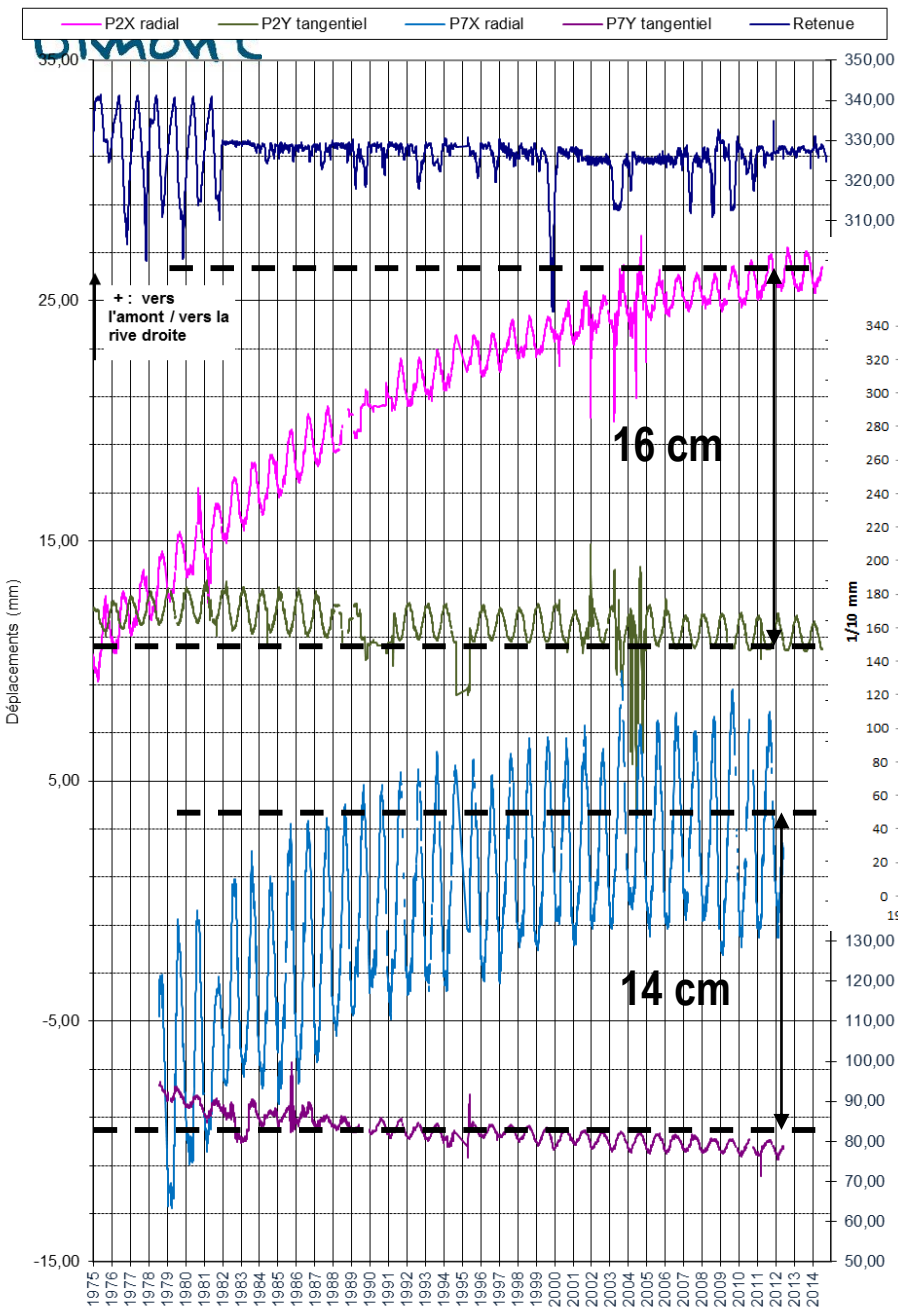
⇒ A geological cause of the damages was rejected



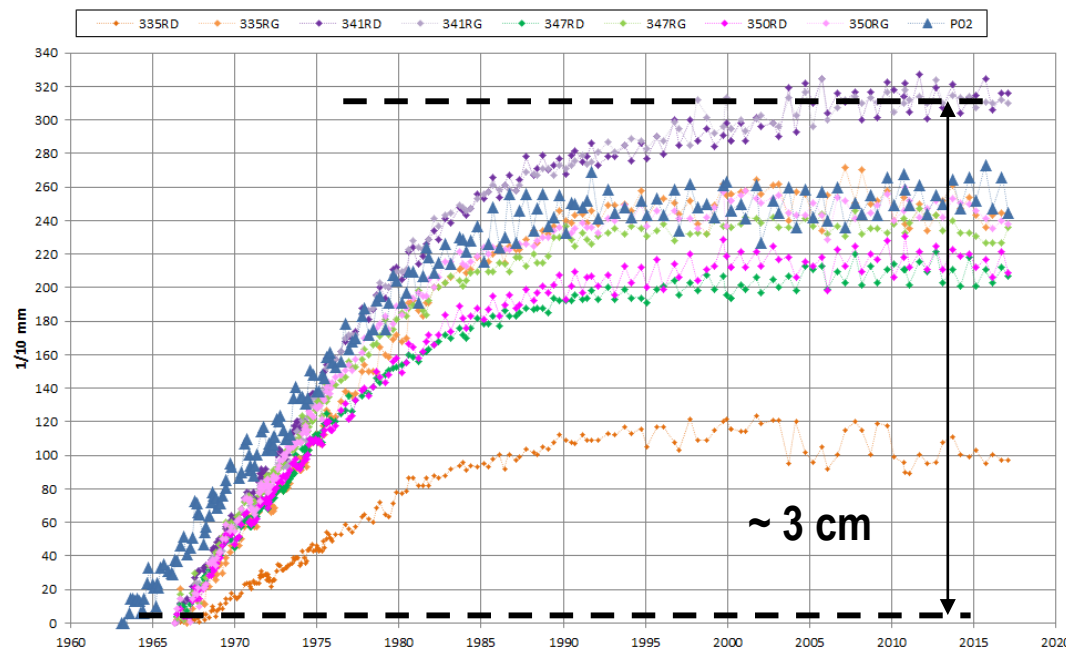
BARRAGE DE BIMONT Représentation des mouvements radiaux et tangentiels au 17/09/2014 x2000



Télépendules - Mesures automatiques

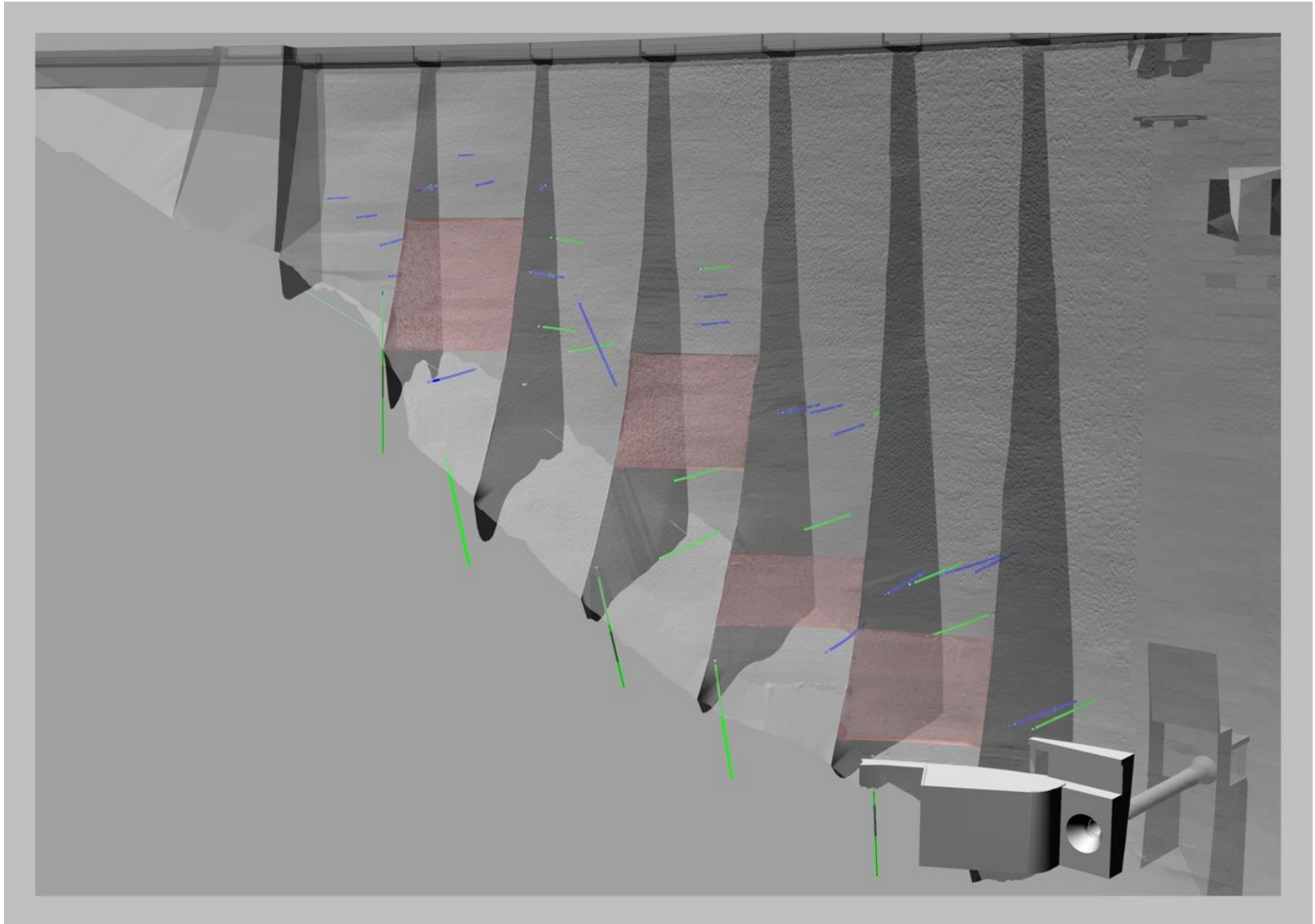


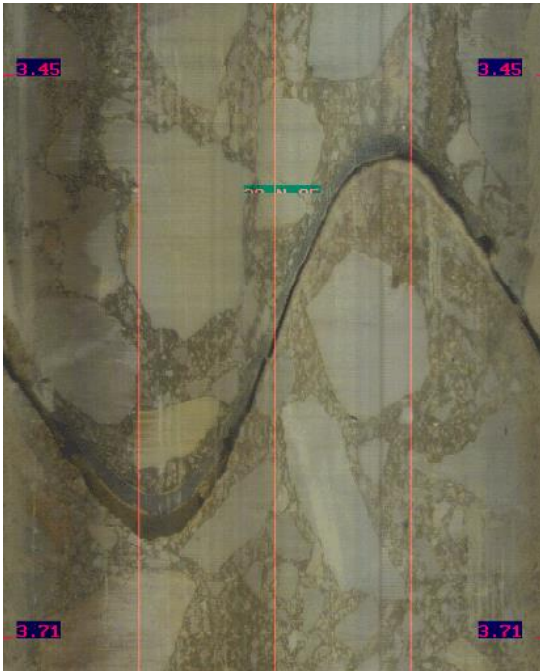
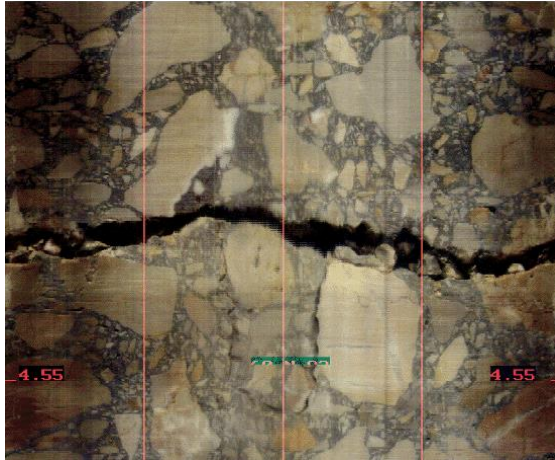
Levelling cantilever 2



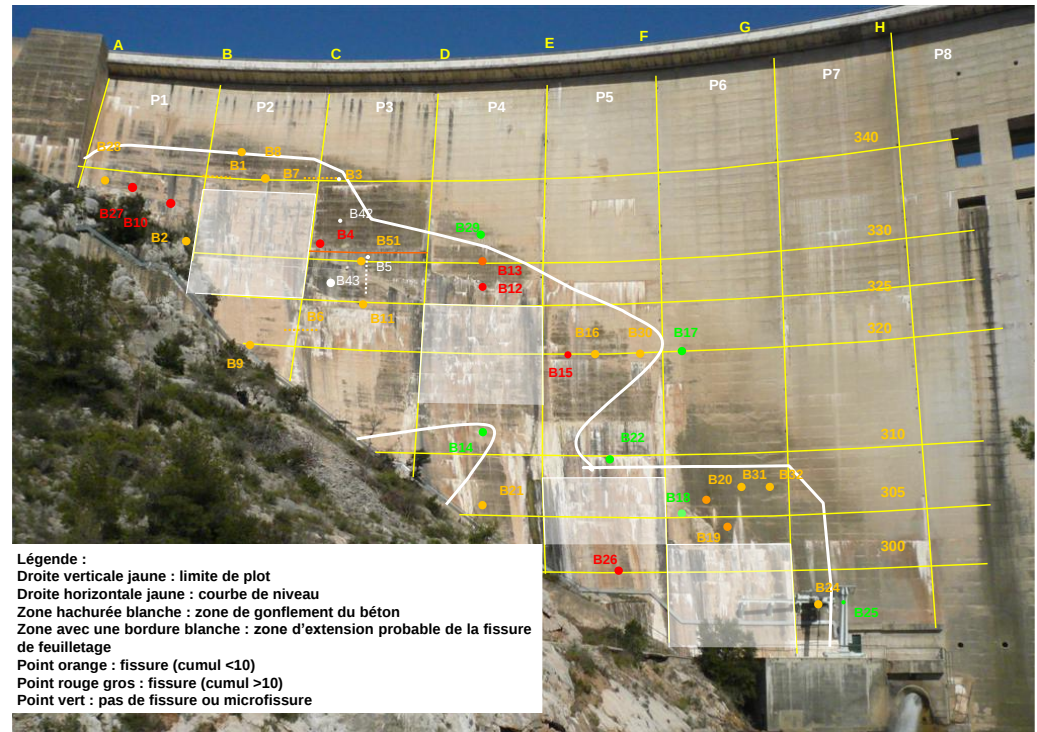
02

PRELIMINARY STUDIES





⇒ The investigations showed internal cracks into the seven cantilevers of the rights banks





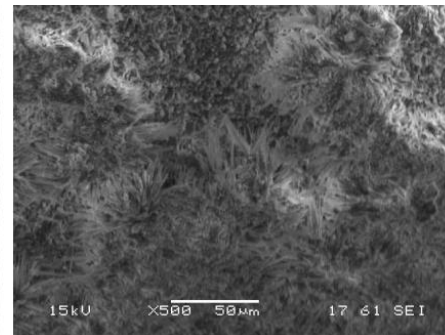
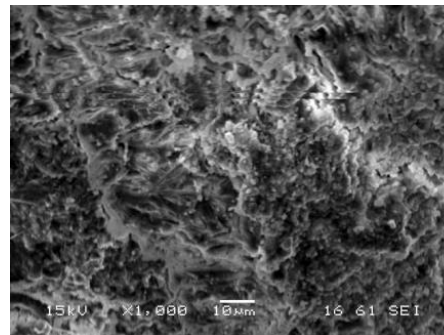
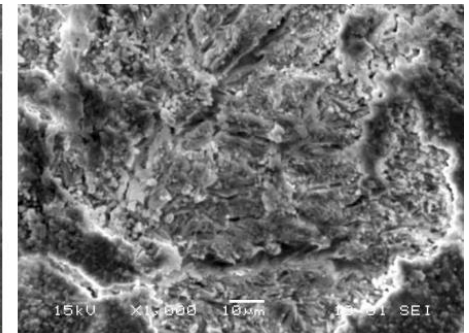
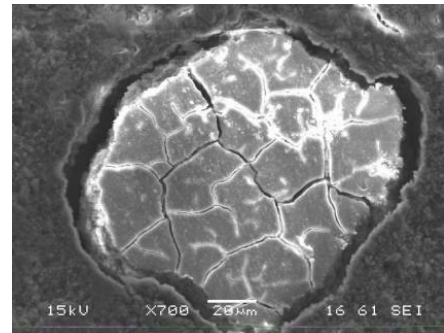
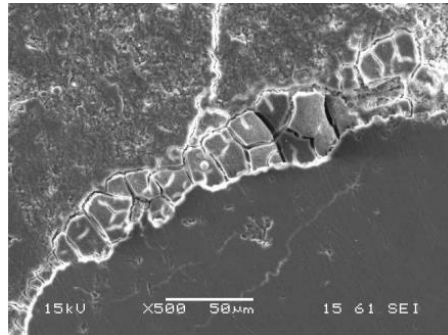
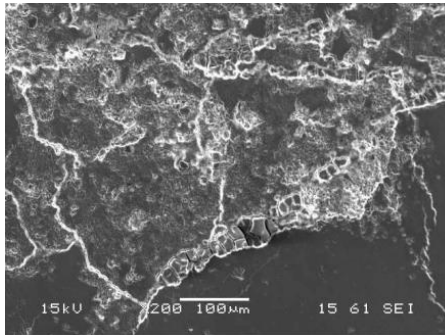
Division Transfert de Technologie

Laboratoire Matériaux et Durabilité des Constructions

INSA/UPS - Génie Civil - 135 Avenue de Rangueil - 31077 Toulouse Cedex 04

Tel : 05.61.55.60.08 Fax : 05.61.55.81.49 mail : lmdc-dtt@insa-toulouse.fr

- Electron microscope, energy dispersive X-ray spectrometer
⇒ Presence of massive secondary ettringite resulting from an internal sulphate reaction.





Division Transfert de Technologie

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INSA/UPS - Génie Civil - 135 Avenue de Rangueil - 31077 Toulouse Cedex 04

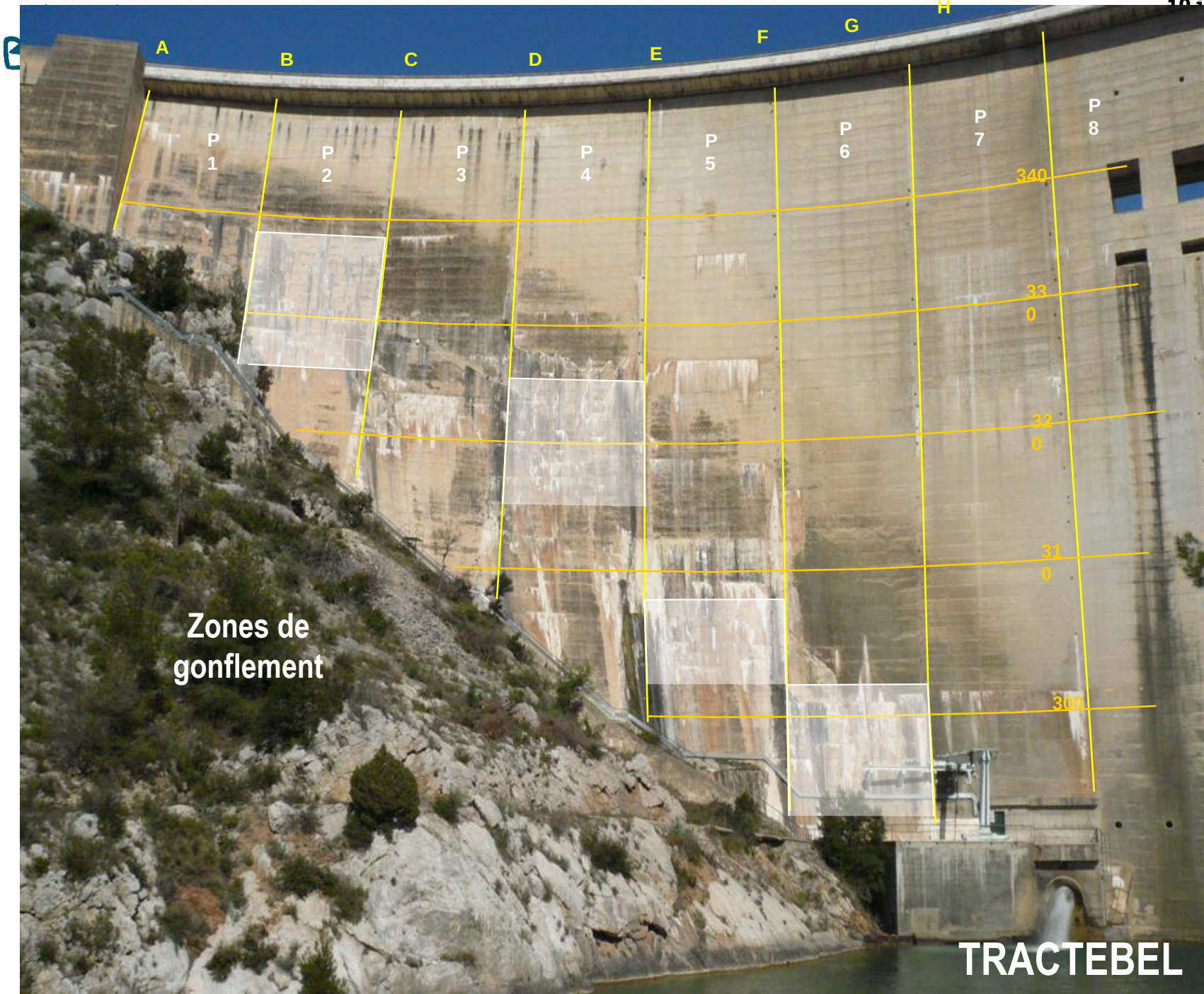
Tel : 05.61.55.60.08 Fax : 05.61.55.81.49 mail : lmdc-dtt@insa-toulouse.fr

- Test performed during 106 weeks to assess the residual expansion of concrete samples taken into sane concrete and swelling concrete.

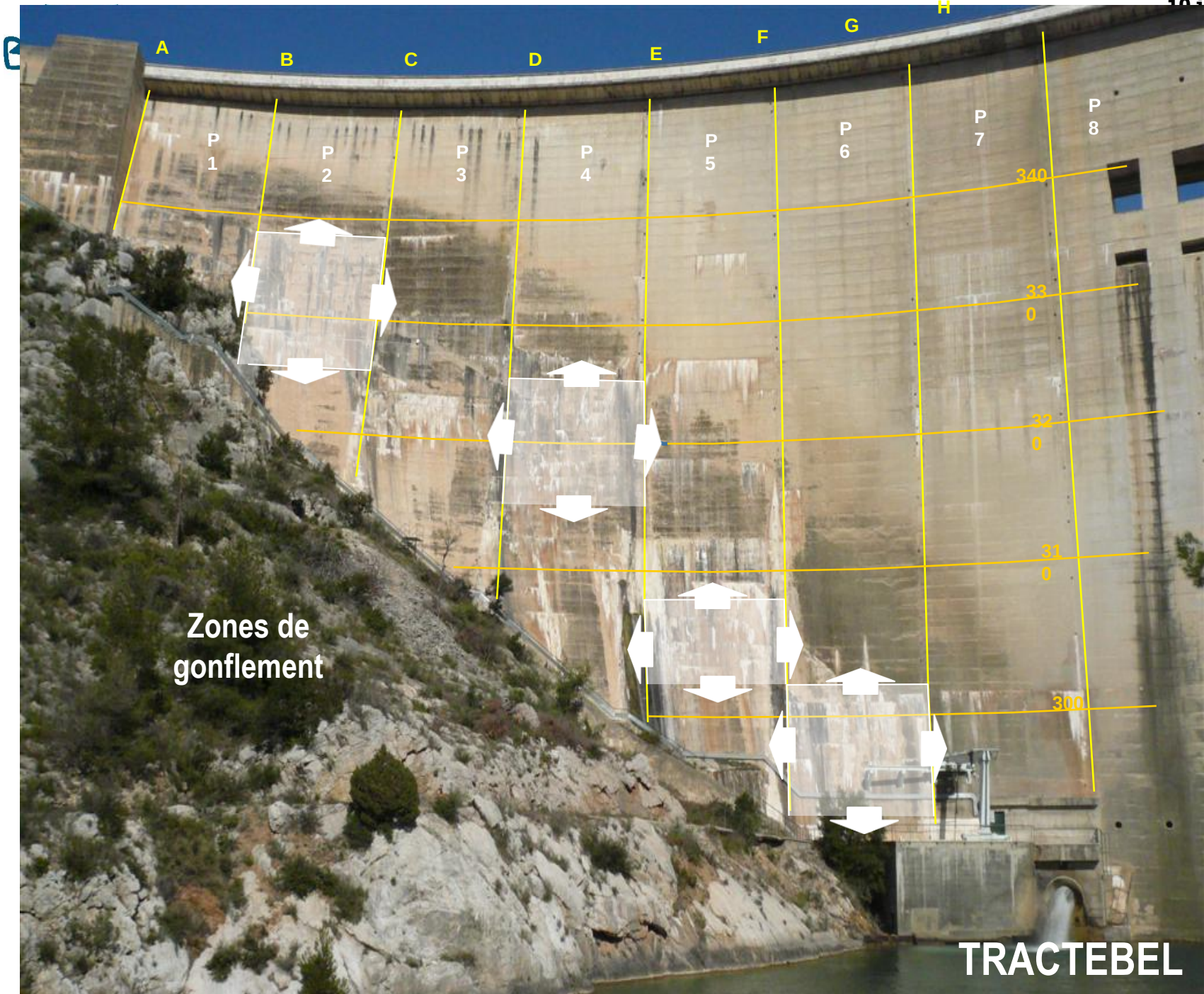
S1		
1,70 à 1,90 mm	3,50 à 3,70 mm	5,00 à 5,20 mm
60	98	171
S2		
1,40 à 1,60 mm	3,60 à 3,80 mm	4,80 à 4,95 mm
72	88	94
S3		
1,65 à 1,85 mm	3,50 à 3,70 mm	5,10 à 5,30 mm
54	84	82
S4		
1,65 à 1,85 mm	3,25 à 3,45 mm	4,70 à 4,90 mm
306	129	155
B13		
1,50 à 1,70 mm	3,50 à 3,70 mm	5,15 à 5,35 mm
35	25	31

03

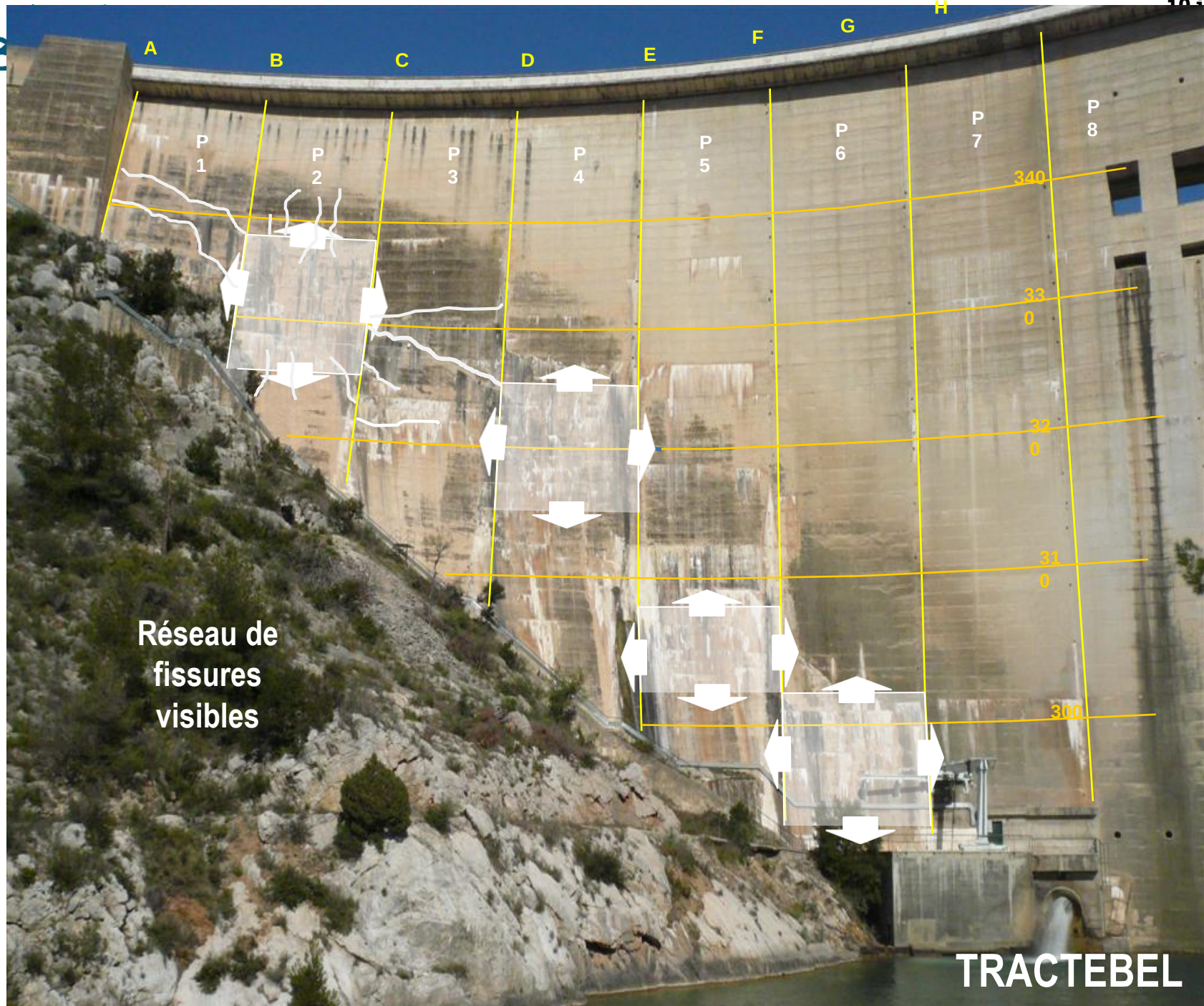
NUMERICAL MODELLING



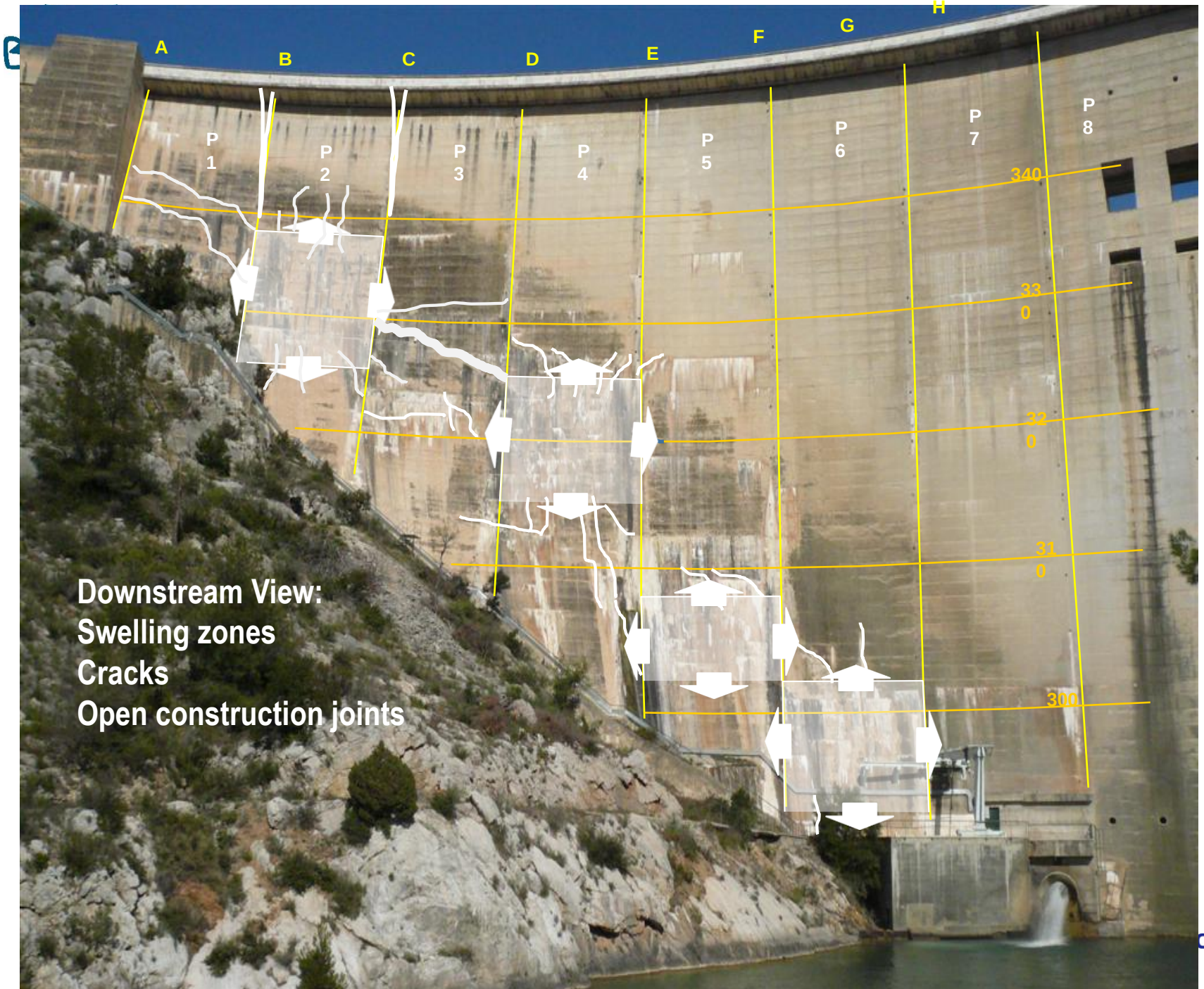
Zones de
gonflement



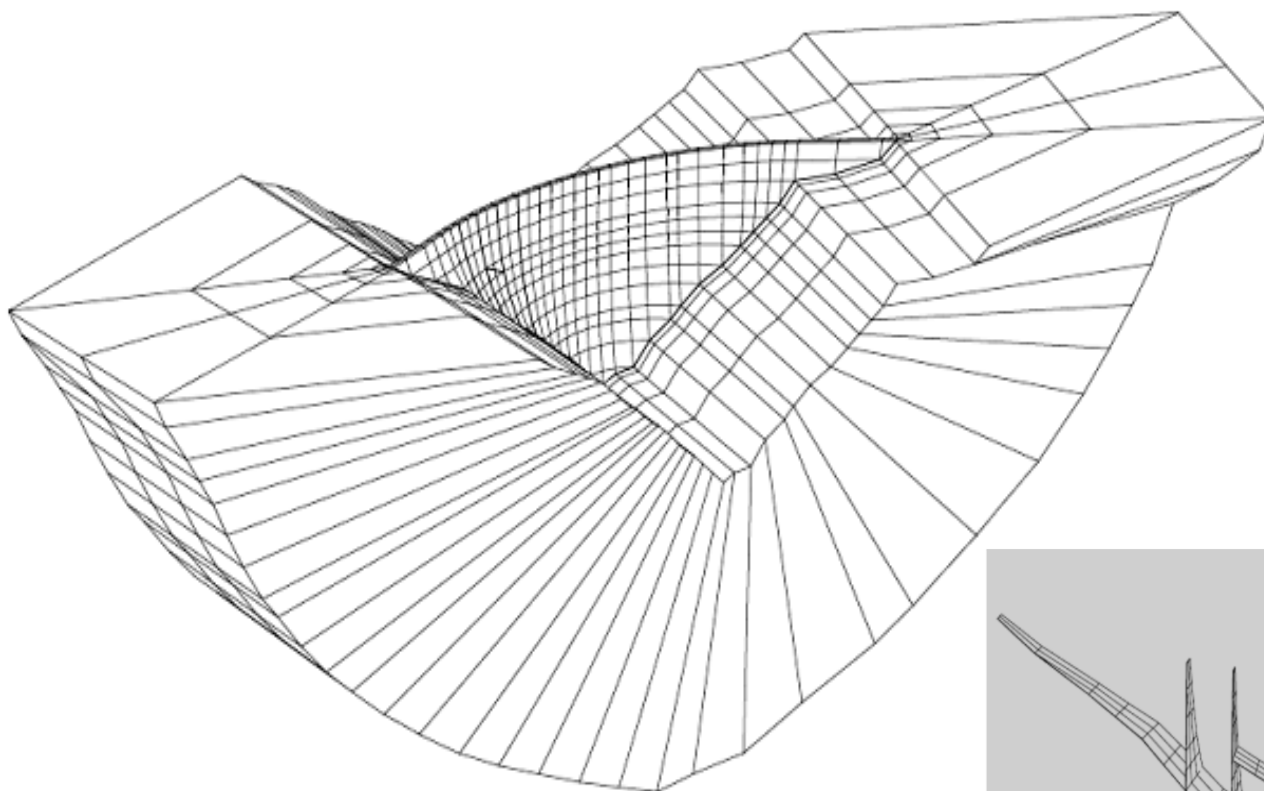
Zones de
gonflement



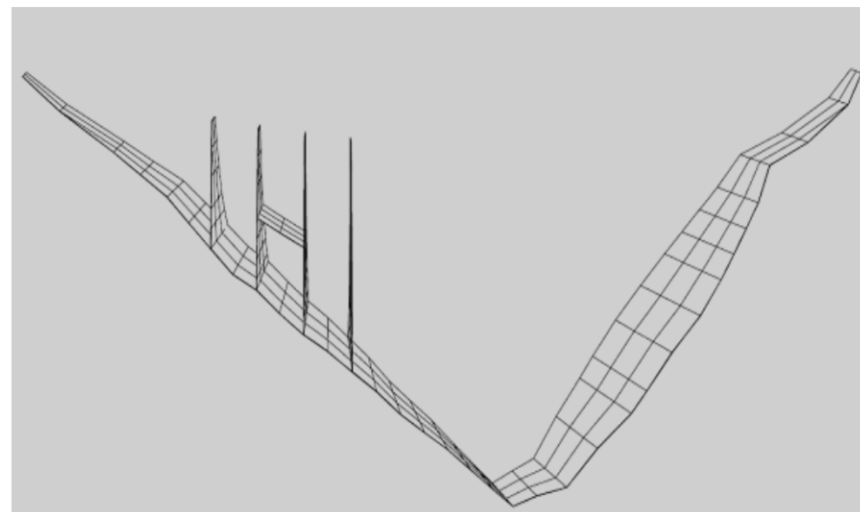
Réseau de
fissures
visibles



Downstream View:
Swelling zones
Cracks
Open construction joints



0 100



- Isotropic thermal equivalence

$$\lambda \times \Delta\theta = \varepsilon_g = \dot{\varepsilon}_g \times \Delta t$$

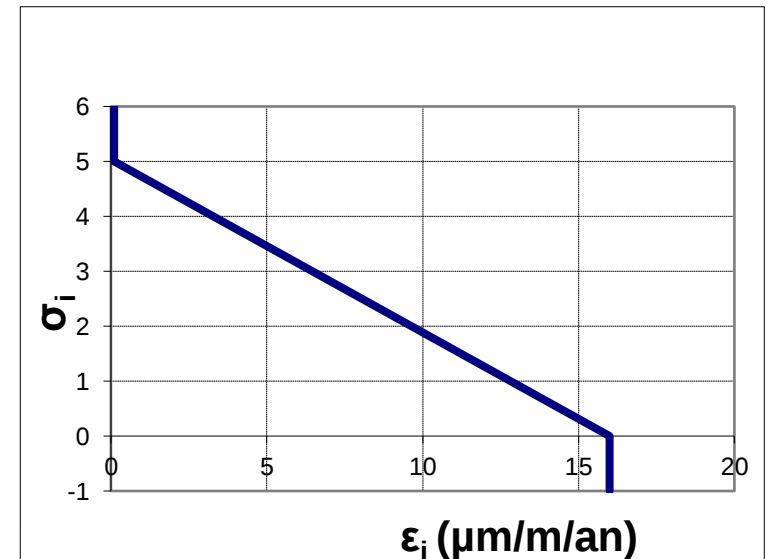
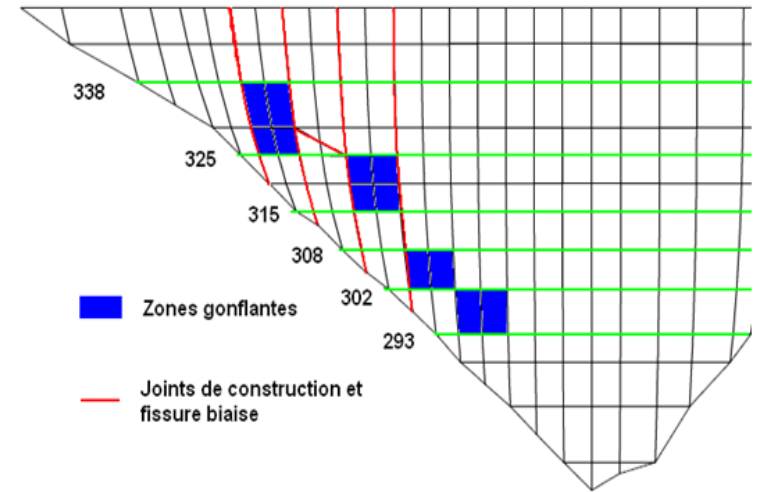
- SCAS model (Stress Controlled Anisotropic Swelling) :

$$\dot{\varepsilon}_g(i) = F(\sigma_i) \quad \text{with } i=1, 2, 3$$

$$F(\sigma_i) = 0 \quad \text{if } \sigma_i \geq \sigma_L \text{ (high compression),}$$

$$F(\sigma_i) = \dot{\varepsilon}_{gL} \quad \text{if } \sigma_i \leq 0 \text{ (tension),}$$

$$F(\sigma_i) = \dot{\varepsilon}_{gL} \times \left(1 - \frac{\sigma_i}{\sigma_L}\right) \quad \text{si } 0 \leq \sigma_i \leq \sigma_L$$

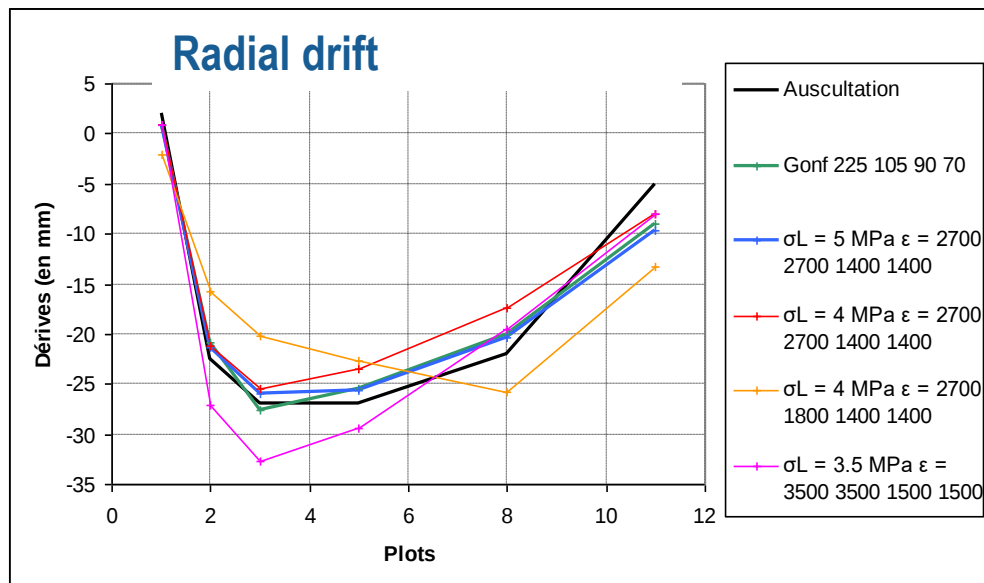
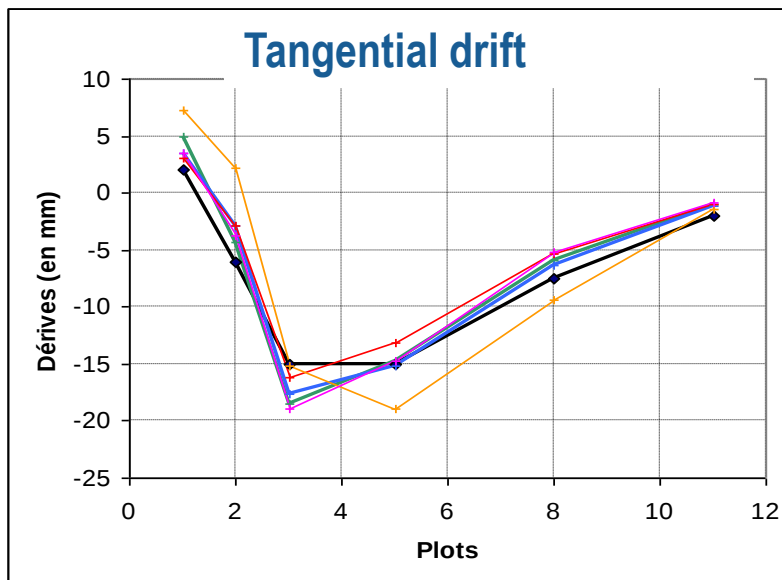
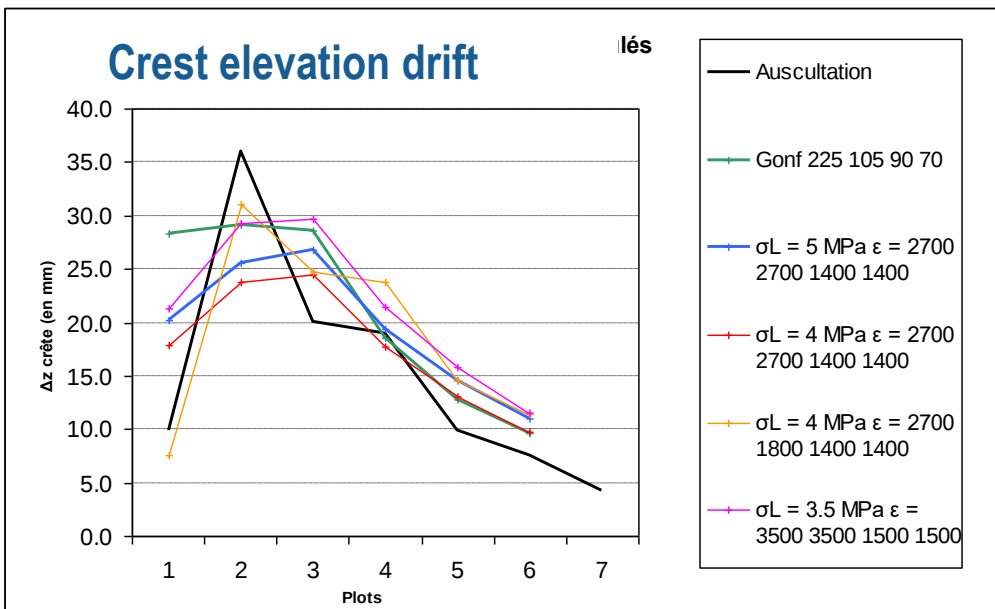


STRESS CONTROL ANISOTROPIC SWELLING LAW (SCAS)

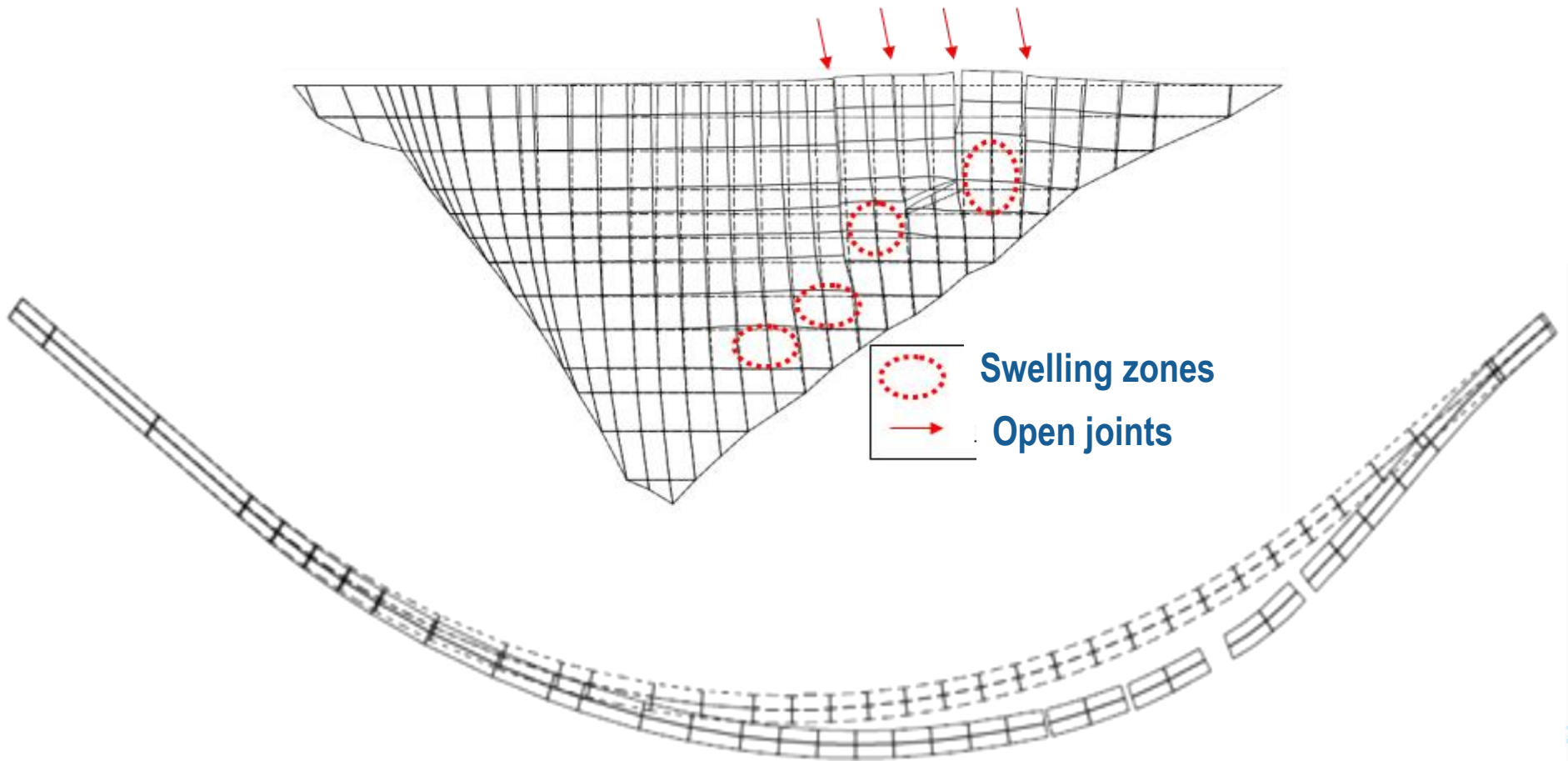
WITH

$$\sigma_L = 5 \text{ MPa}, \dot{\varepsilon}_{GL} = 90 \cdot 10^{-6} \text{ M/M/Y}$$

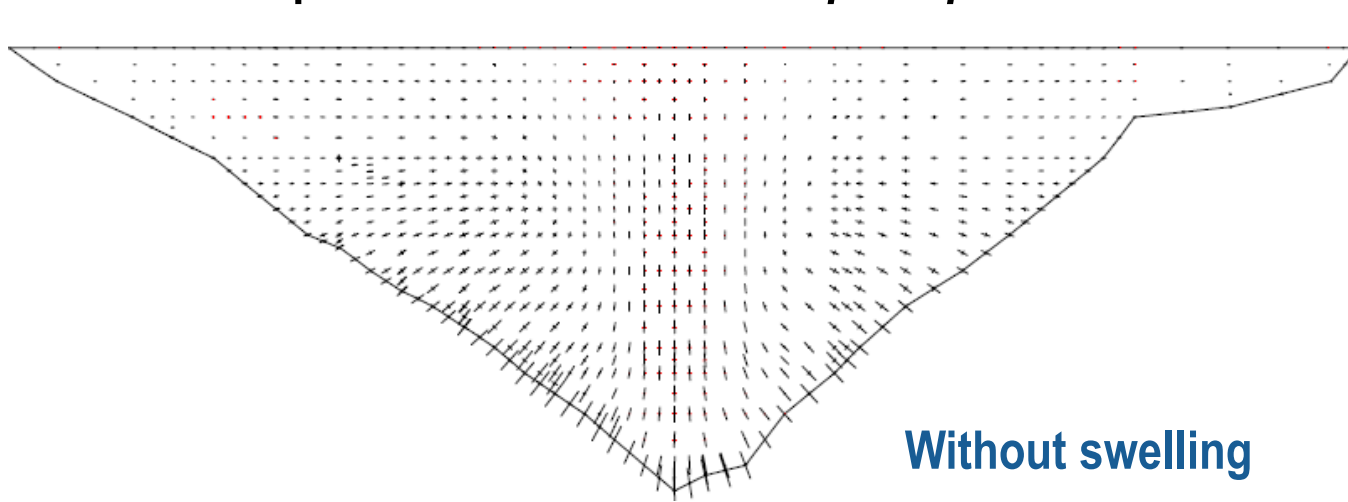
ε_{GL} varying from
1,4 to 2,7 mm/m



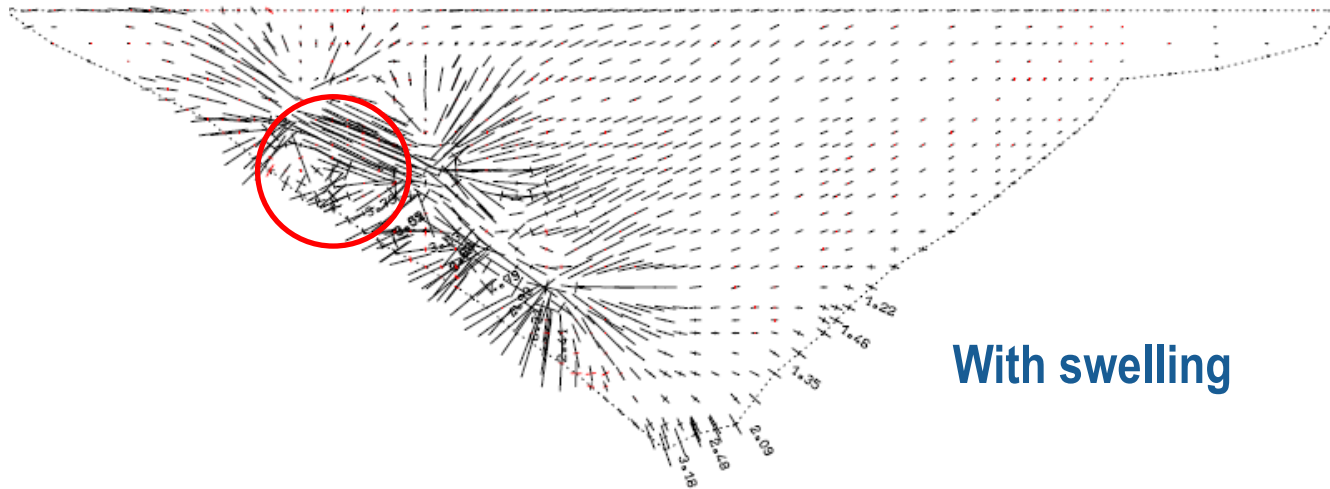
Deformations



Downstream face / *parement aval*
Principal stresses / *contraintes principales*

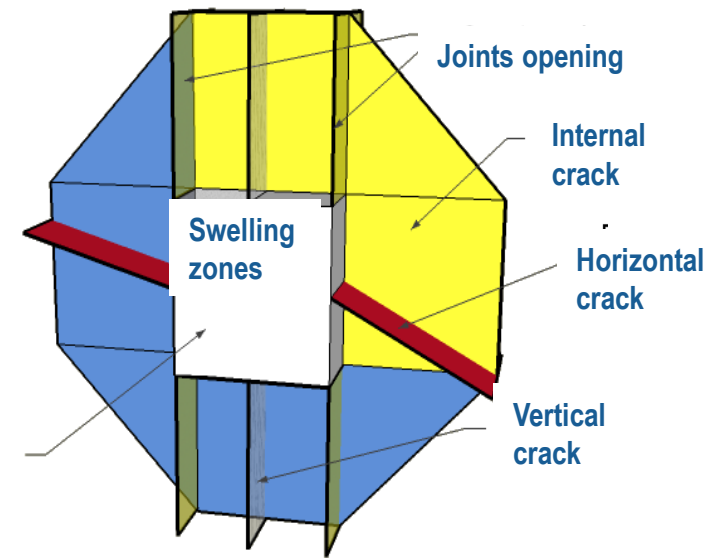
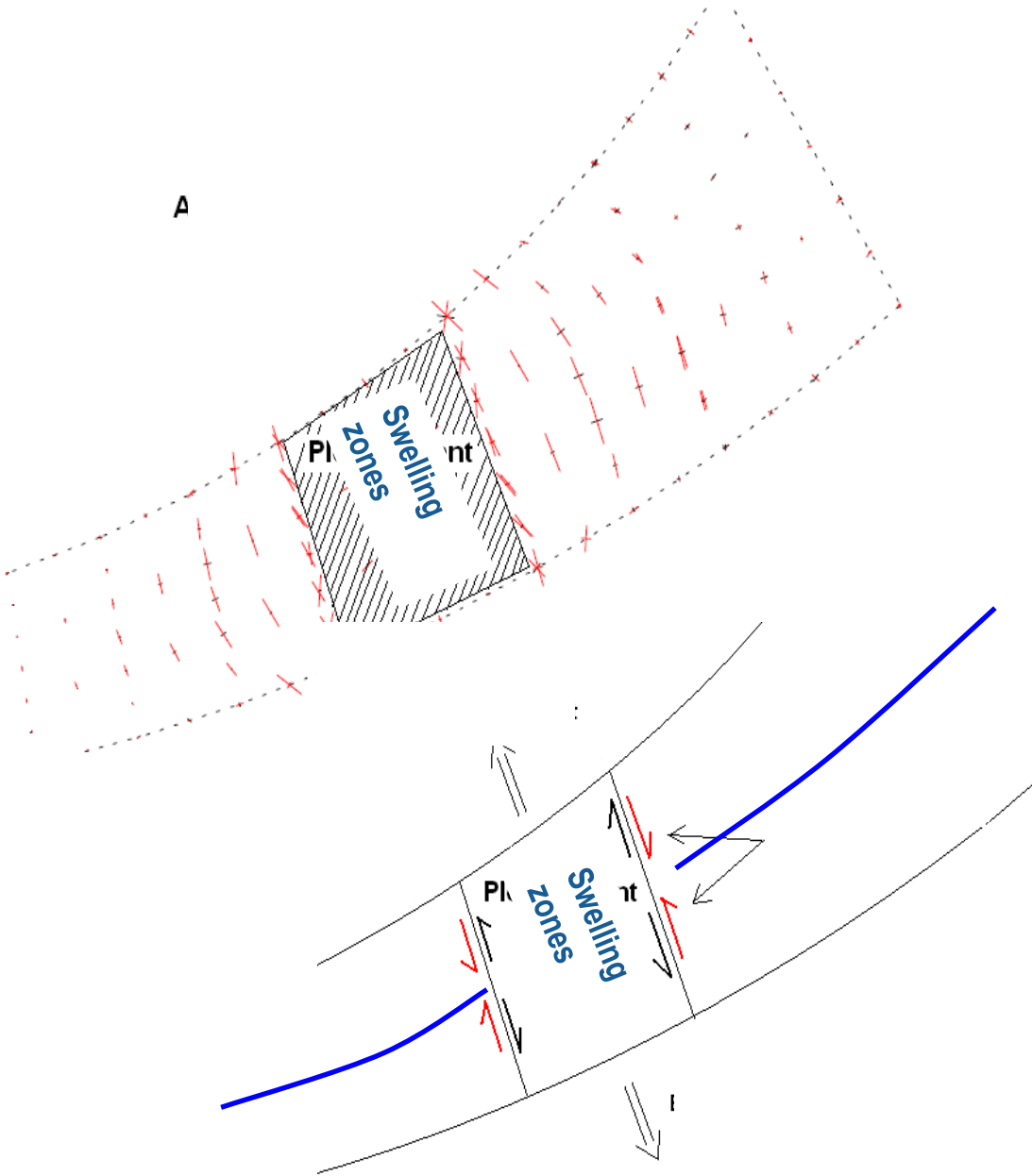


Without swelling

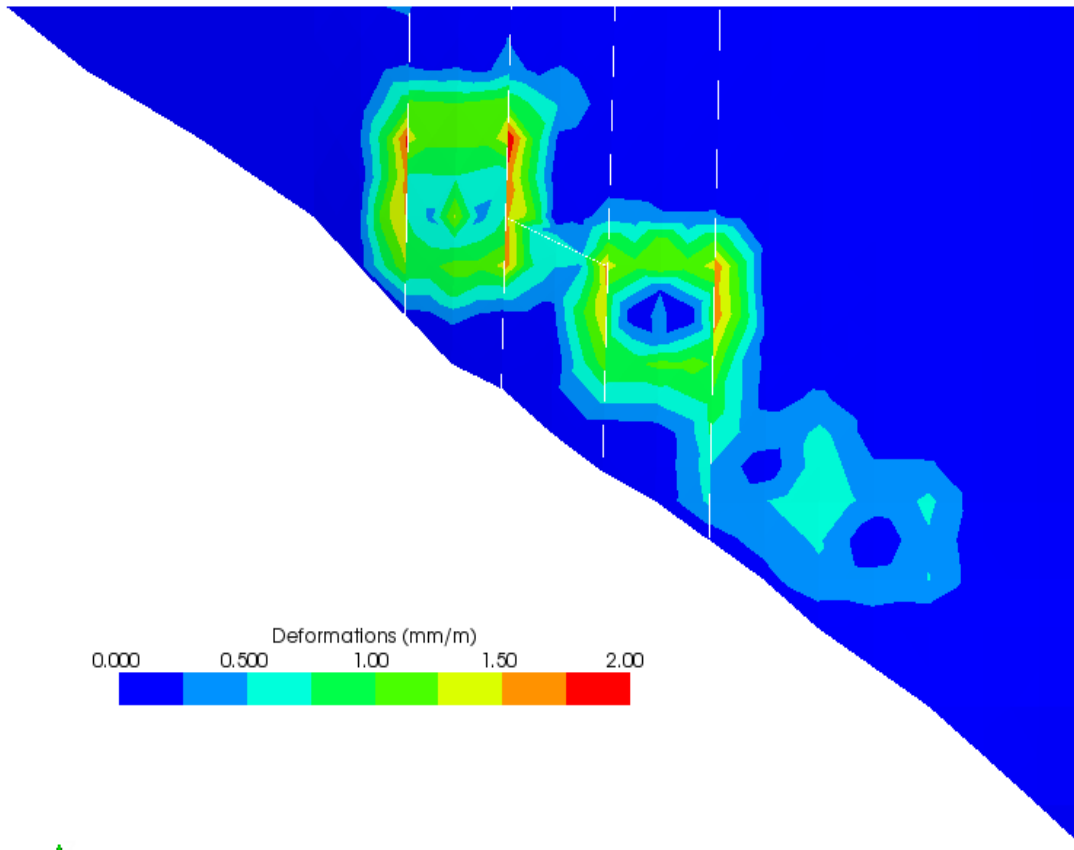


With swelling

Interpretation

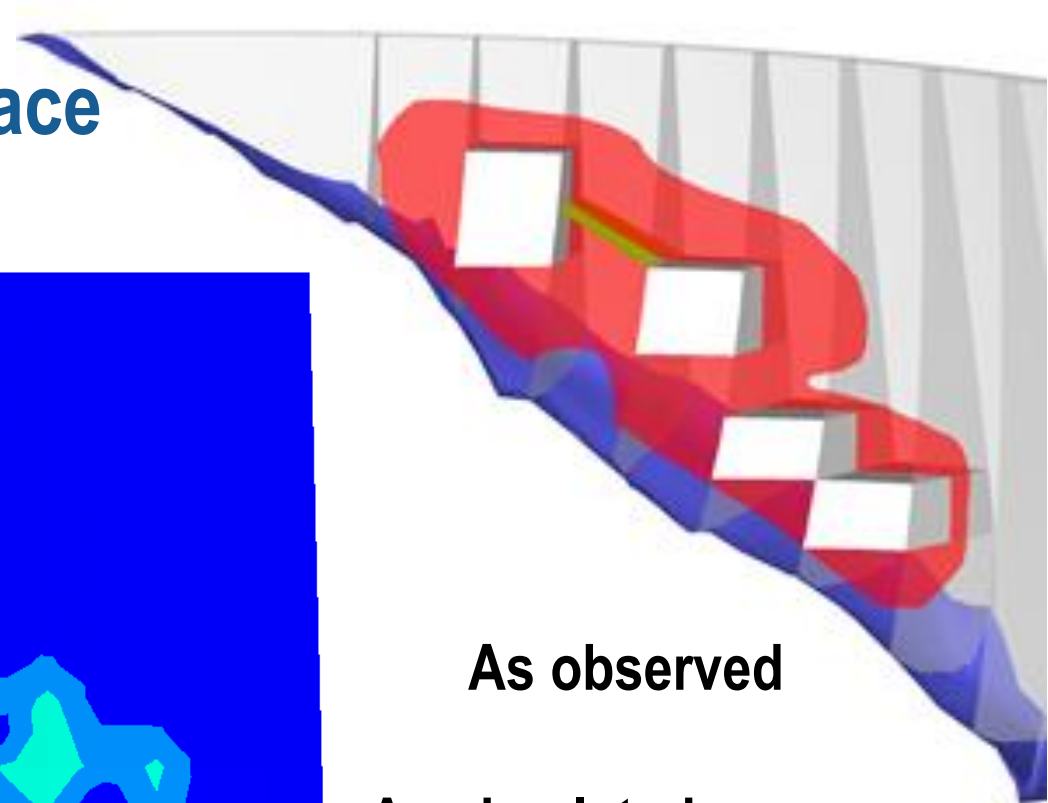


Internal cracking surface



As observed

As simulated



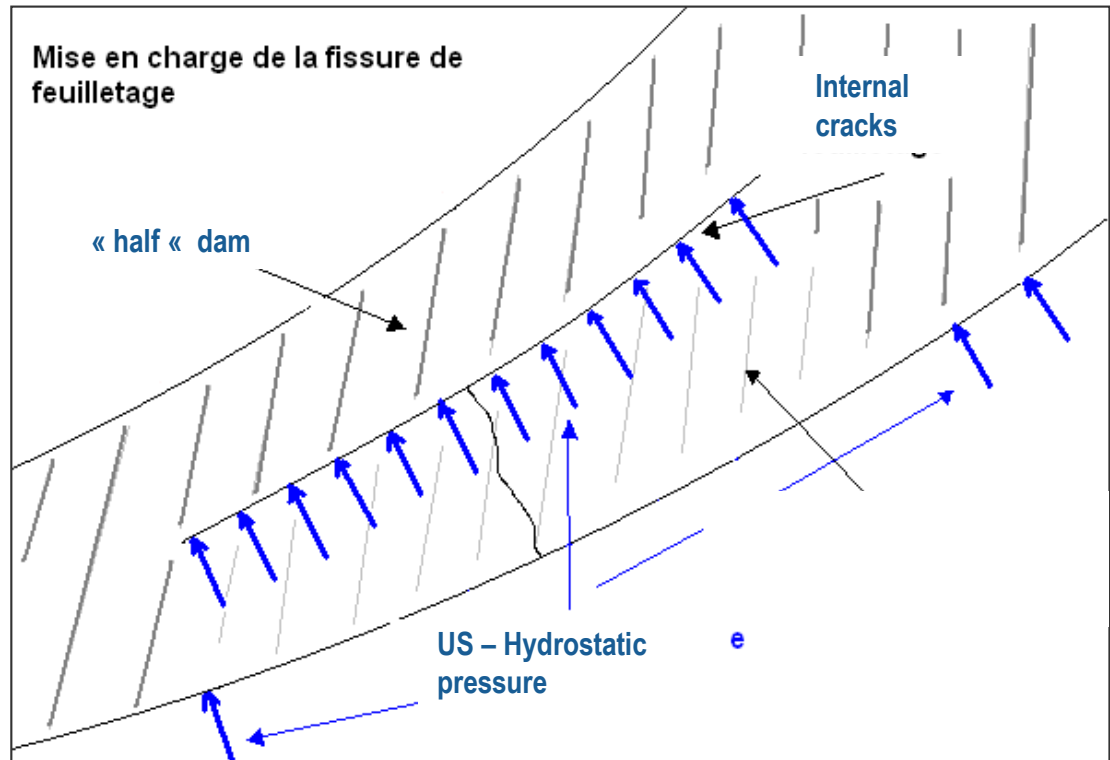
04

REHABILITATION WORKS

- Cracks grouting
- Construction joints grouting
- Upstream face watertightning
- RB thrust block reinforcement
- Drainage curtain

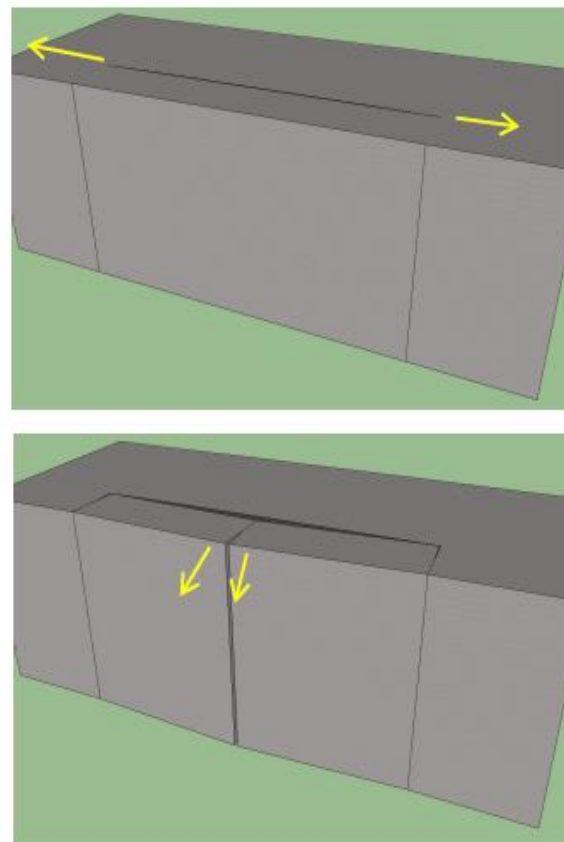
Cracks grouting: why ?

- Avoid application of pore pressure on internal cracks surface
- allow the arch stresses to be transmitted in the dam body



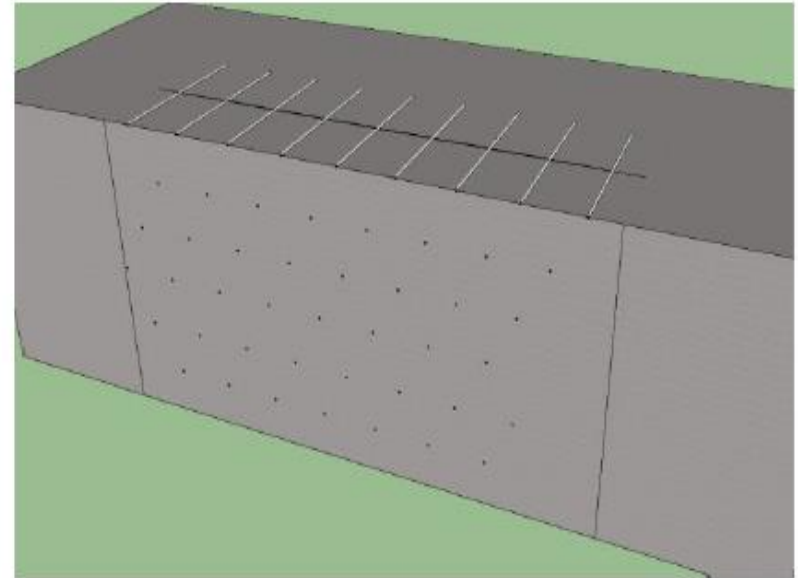
Cracks grouting

- Identified risks
 - Potential extension of cracks under grouting pressure
 - Possibility to eject the concrete part between the internal crack and dam faces due to grouting pressure
 -

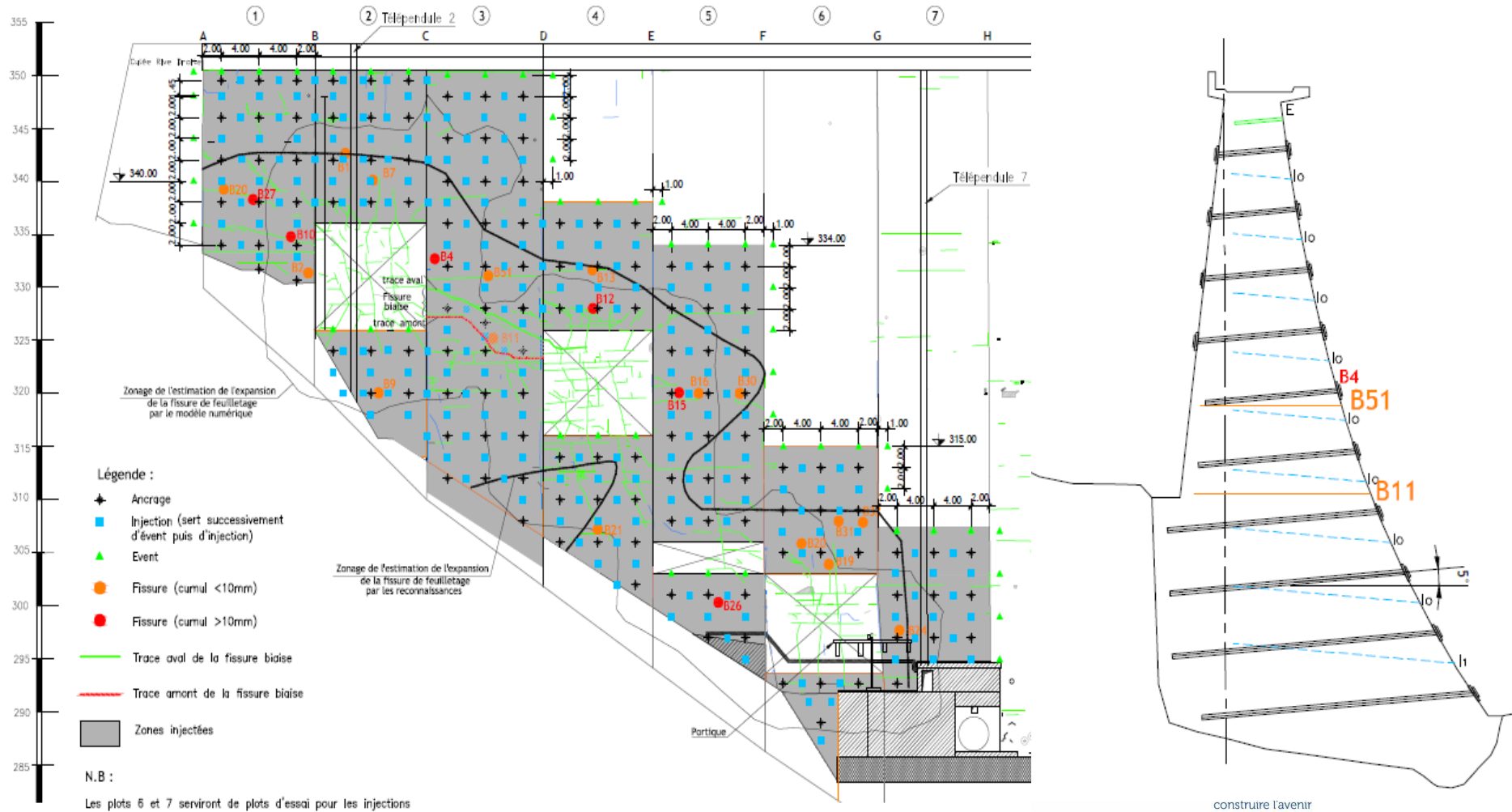


Cracks grouting

- Technical solutions
 - Temporary reinforcement with passive horizontal anchors
 - Systematic inspection of all drillings by camera
 - Cement grouting process bottom up
 - Grouting pressures carefully monitored on reduced surfaces (4 x 4 m)
 - Automatic topographic surveillance

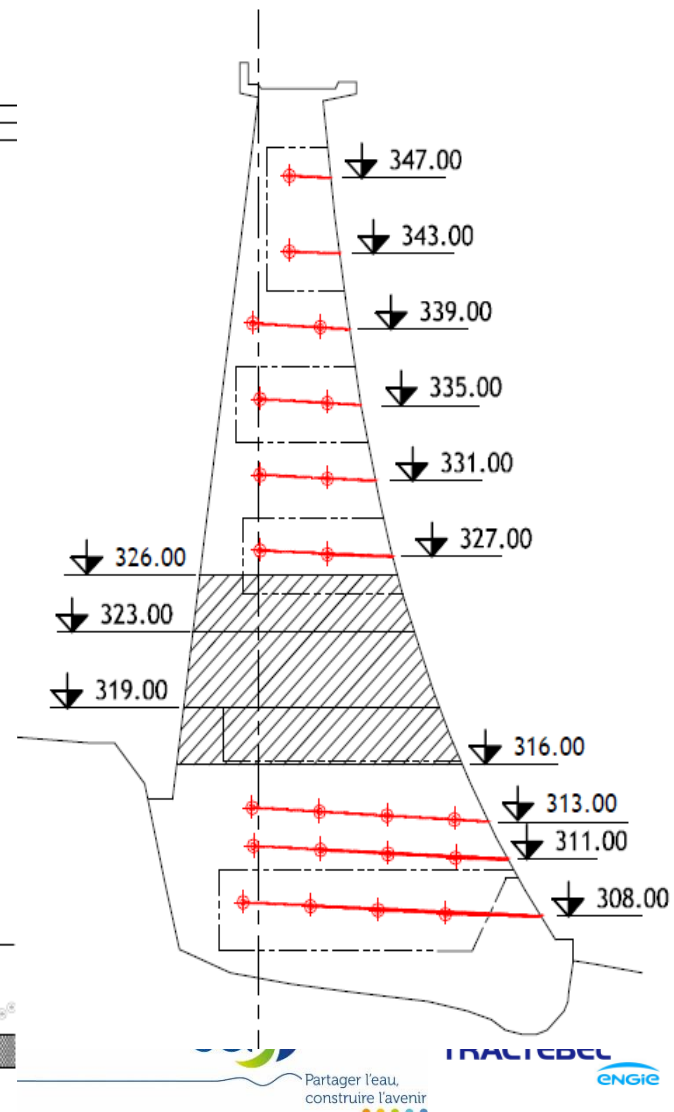
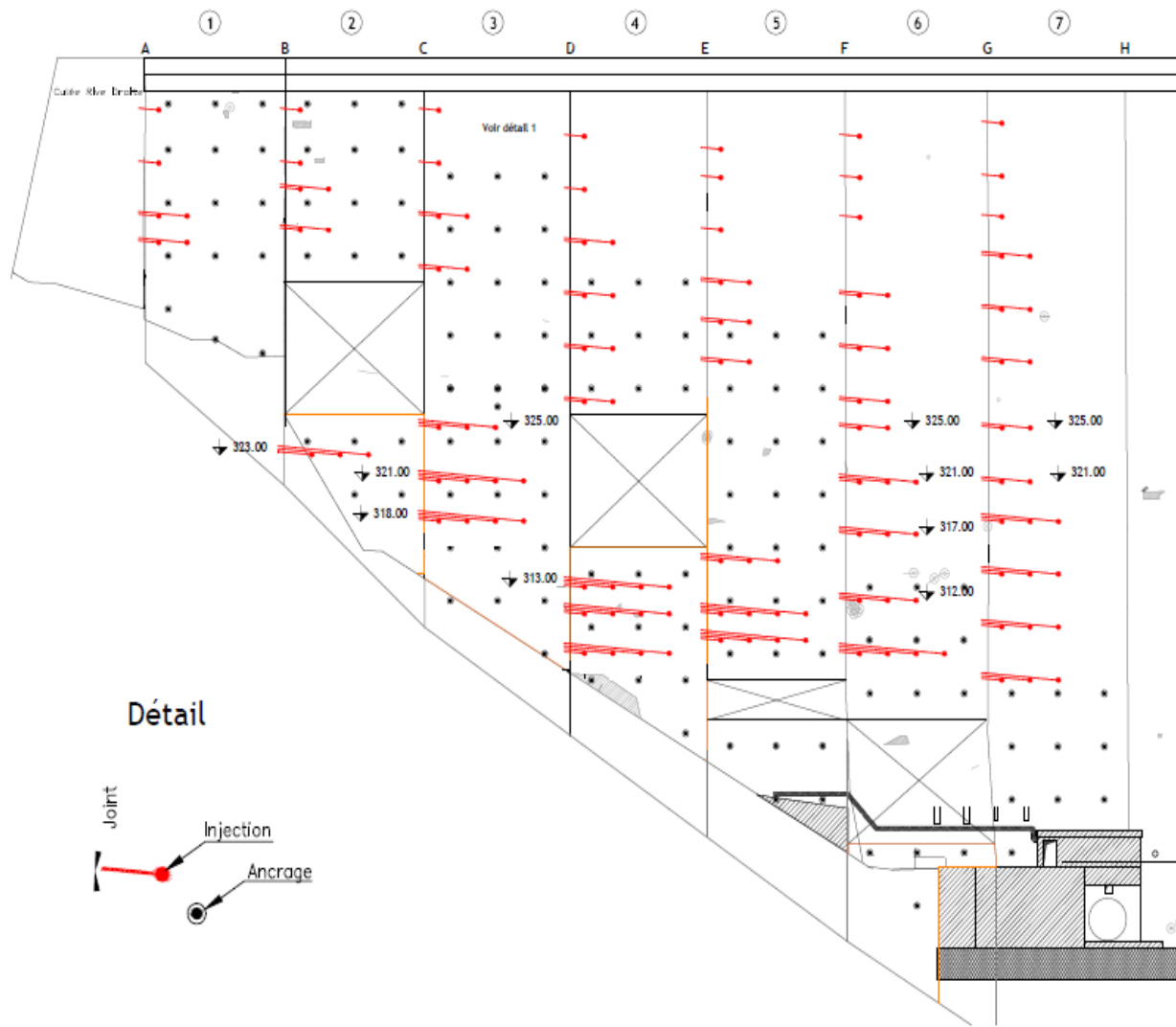


Cracks grouting : passive anchors implementation

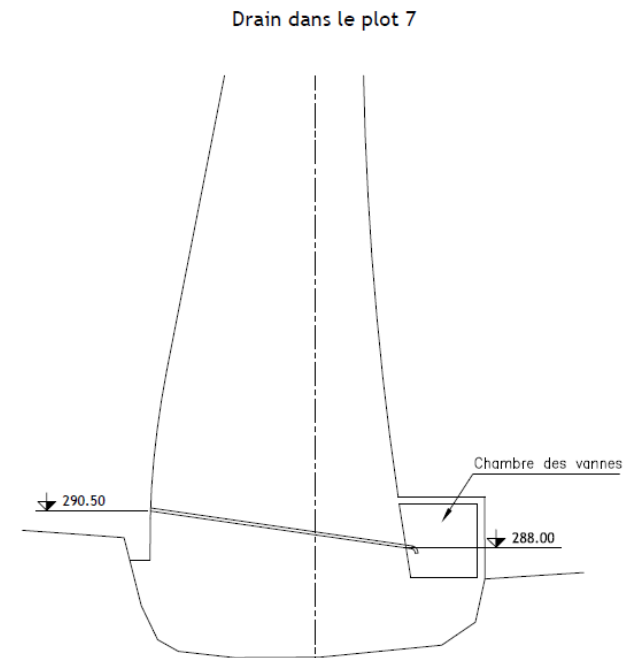


Construction joints grouting

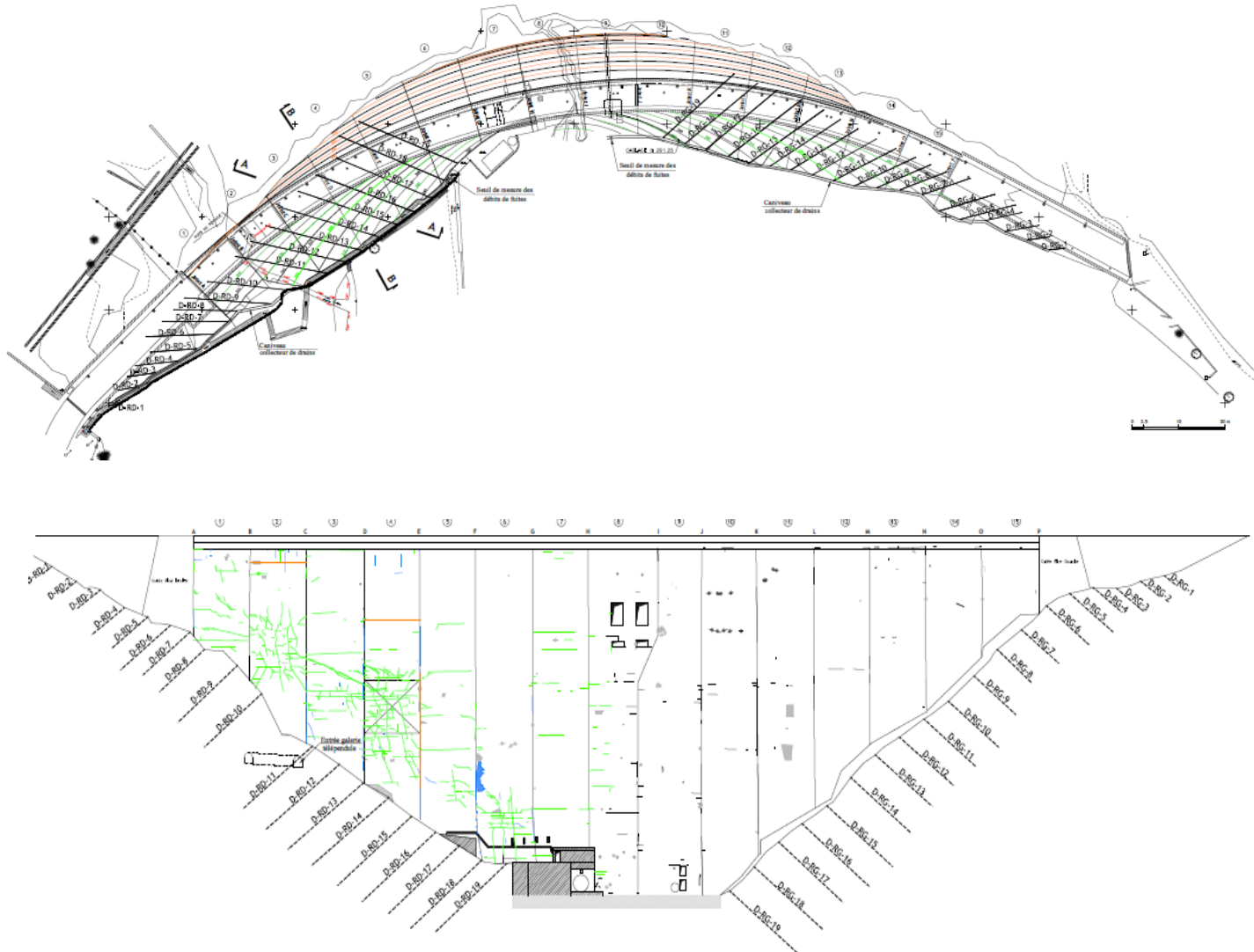
Coupe sur le joint D



US face watertighting



Drainage curtain in the foundation



**1 – Water by-pass implementation
Reinforcement of monitoring system**

2 – Reservoir emptying

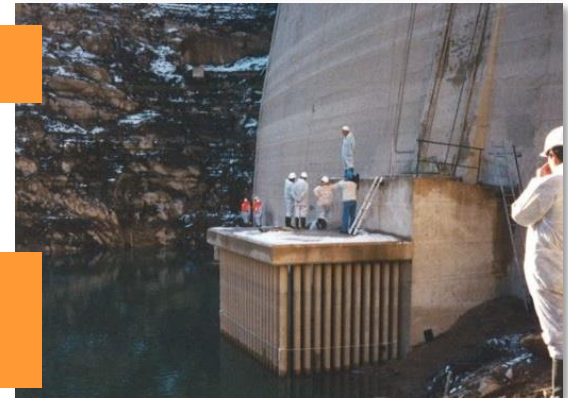
3 – Rehabilitation works

4 – Reservoir refilling

2016



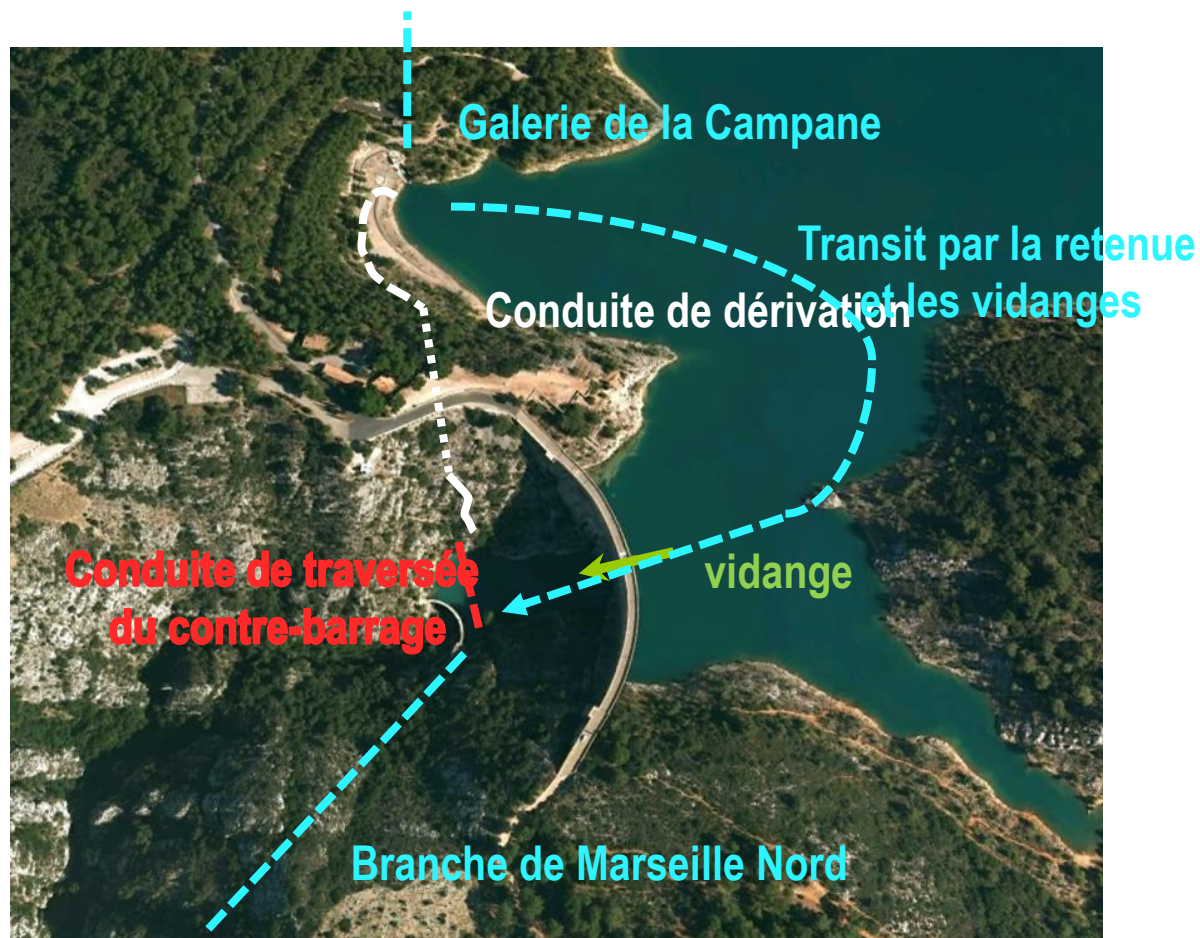
2017



2017
2019

2019



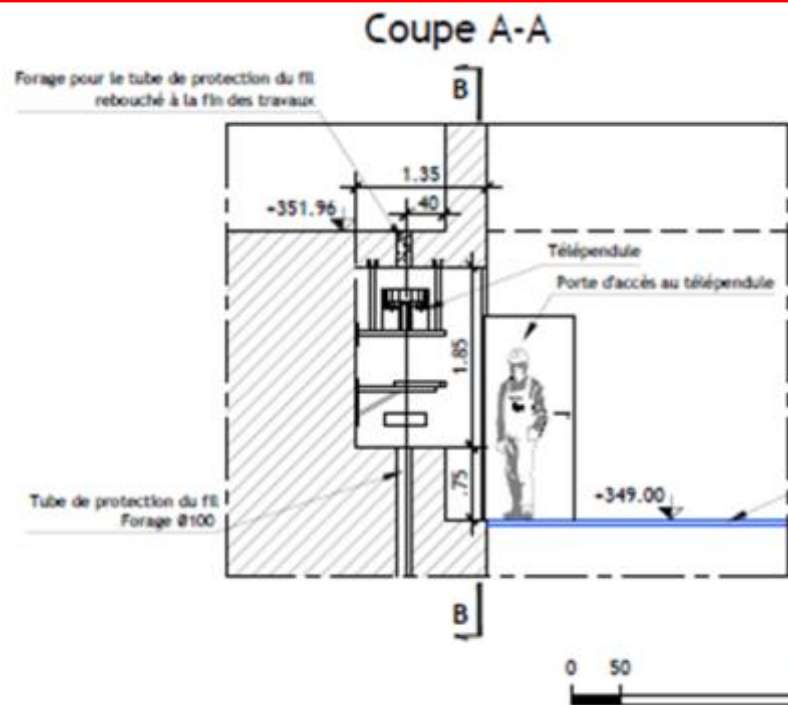
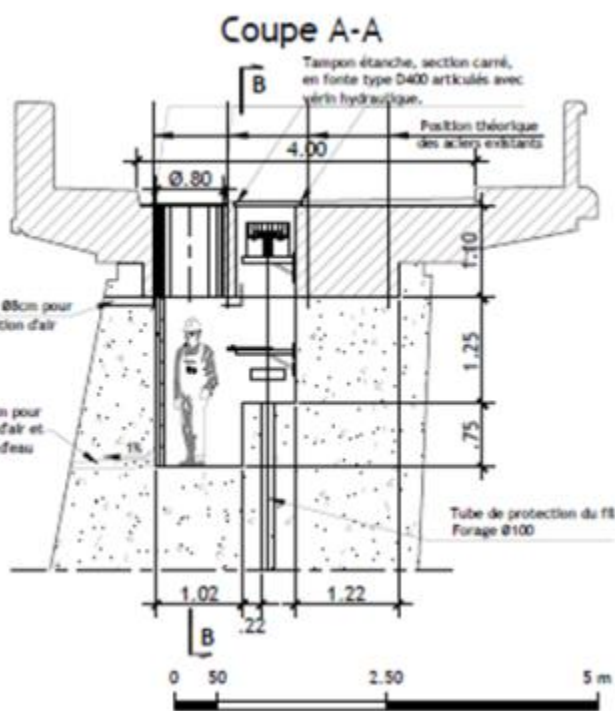
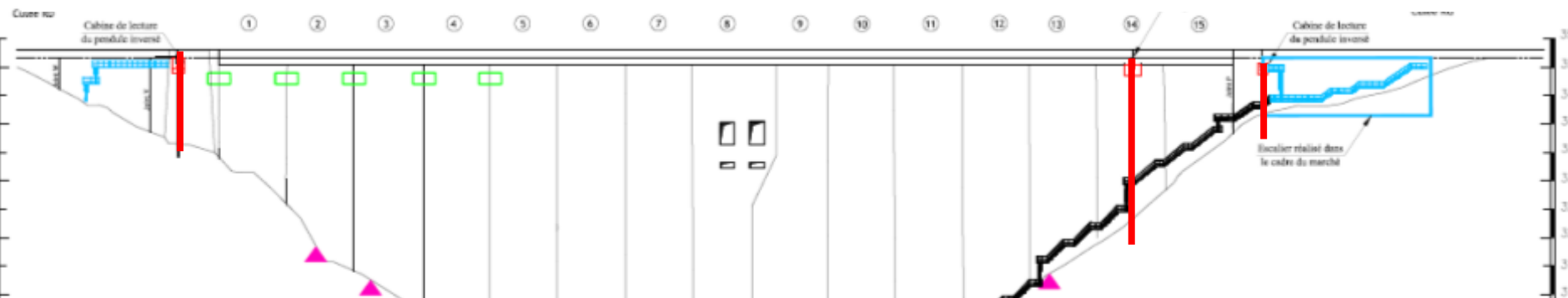


21/11/2017



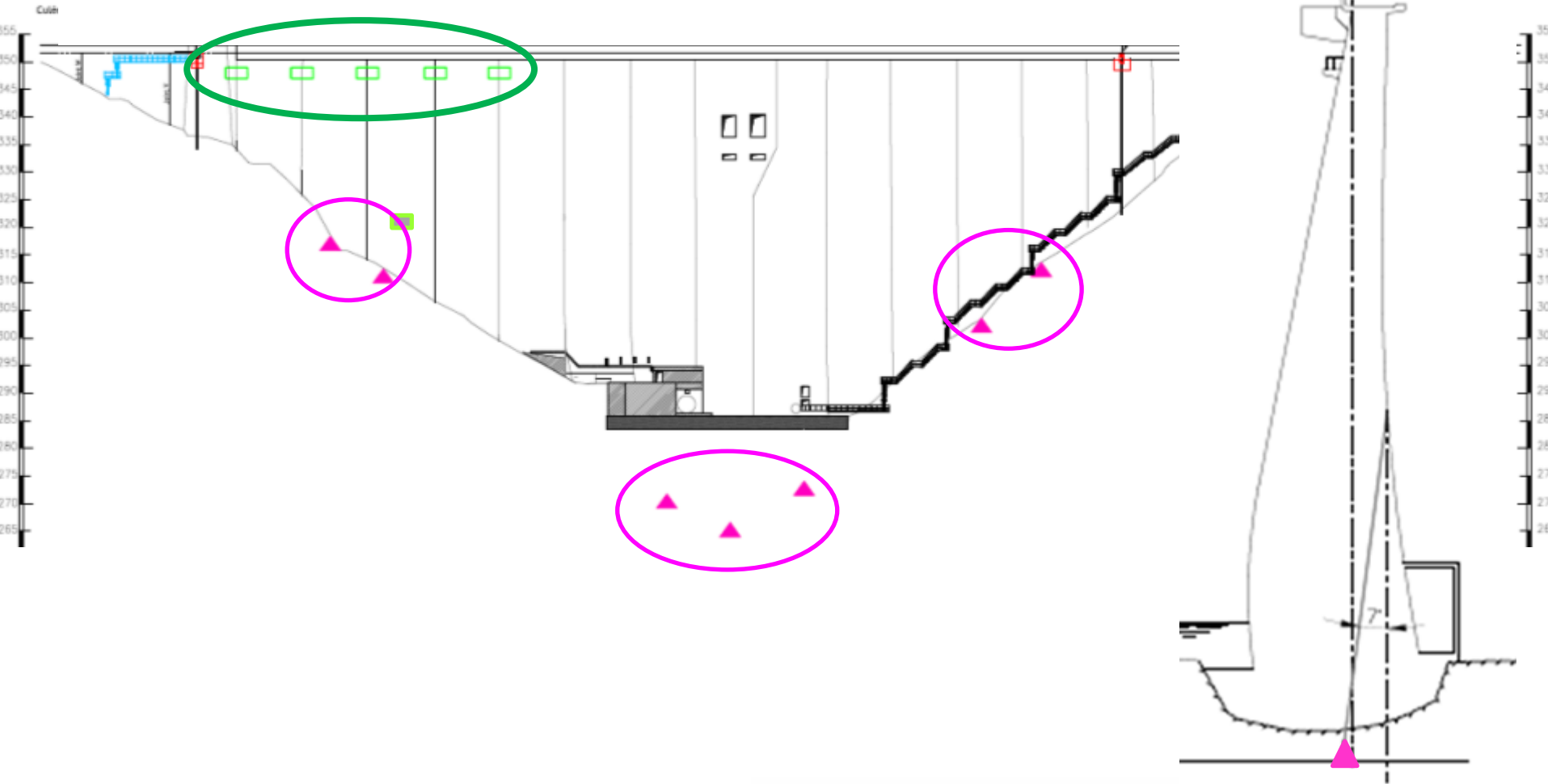
LA RENOVATION DU DISPOSITIF D'AUSCULTATION

➔ 3 nouveaux télépendules, inversés

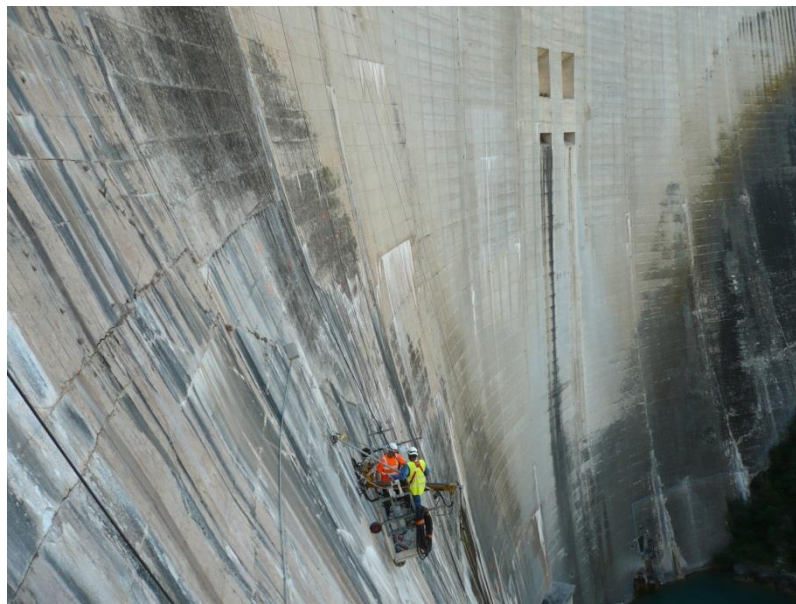


LA RENOVATION DU DISPOSITIF D'AUSCULTATION

- ➔ 5 téléfissuromètres
- ➔ 7 cellules de pression interstitielle



LA RENOVATION DU DISPOSITIF D'AUSCULTATION





**THANK YOU FOR YOUR
ATTENTION**